



12-20 Berry Road & 11-19
Holdsworth Avenue
St Leonards South NSW 2065
Australia
Development Application
Stormwater Management Report

FOR / Civil Engineering Services

CLIENT / Aqualand St Leonards Development 3 Pty Ltd

DOCUMENT NO / S21268-RPT-CI-0001 REV / C DATE / 16/06/2022

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CONTENTS

1	INTRODUCTION	1
2	EXISTING SCENARIO	1
2.1	Existing Scenario	1
2.2	Existing Stormwater Pipe Infrastructure	3
2.3	Existing Site Drainage Description	3
3	PROPOSED DEVELOPMENT	4
4	DESIGN CRITERIA	6
4.1	Stormwater Policies and Objectives	6
4.2	Proposed Drainage Design - Overview	6
4.3	Existing Development	6
4.4	Proposed Drainage Design – Building	8
4.5	Proposed Drainage Design – Through Site Link	9
4.6	Water sensitive Urban Design	9
5	ANALYSIS RESULTS	9
5.1	On-Site Detention Design	9
5.2	Water Sensitive Urban Design Implementation	9
5.3	Proposed Stormwater Treatment Train	11
5.3.1	Ocean Protect 690 PSORB Stormfilter Cartridges (MCC)	11
5.3.2	Ocean Protect OceanGuard	12
5.3.3	20kL Rainwater Tank	12
6	CONCLUSION	13

Appendices

Appendix A Site Survey

Appendix B Stormwater Plan

Document Control

Revision	Date	Description	Prepared	Reviewed	Approved
A	10/06/2022	Draft	AM	SH	SH
B	15/06/2022	Issued For Approval	AM	SH	SH
C	16/06/2022	Issued For Approval	AM	SH	SH

Relevant Documentation

The following documentation has been used as a reference in the preparation of this report and the stormwater concept plans:

- Architectural Plans by SilvesterFuller June 2022
- Detailed Survey by SDG Land Development Solutions April 2016
- Landscape Concept by RPS Australia East June 2022
- Lane Cove Council Stormwater Concept plan for work in Berry Rd and Holdsworth Ave

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1 INTRODUCTION

BG&E Pty Ltd (BG&E) has been engaged by Aqualand St Leonards Development 3 Pty Ltd (Aqualand) to prepare a Stormwater Management Report for the proposed development of residential apartment buildings at 12-20 Berry Road & 11-19 Holdsworth Avenue St Leonards South NSW 2065 Australia (Site).

This report will outline the methodology adopted and associated results of:

- Requirement of a detention tank; and
- Water sensitive urban design.

It is understood that this report will be utilised in the Development Application submission and will also inform the basis of the next phases of design works to ensure continuity through the project.

2 EXISTING SCENARIO

2.1 Existing Scenario

St Leonards is a suburb on the lower North Shore of Sydney. St Leonards is located 5 km north-west of the Sydney central business district, and approximately 500m from St Leonards Station. The suburb lies across the local government areas of Municipality of Lane Cove, North Sydney Council and the City of Willoughby. The site is located within the Lane Cove Council. The site is bounded by Berry Road on the Eastern boundary and Holdsworth Avenue on the Western Boundary and consist of 10 separate existing lots.

The site is currently 10 separate residential lots consisting of single dwelling and some attached dual-dwelling houses. Refer to Figure 1 and Figure 2 for images of the existing Site.



Figure 1- Site Aerial Image (Nearmaps, 06.10.2021)



Figure 2 - Site Image – looking from 20 Berry Road to 12 Berry Road (Google Street View, Mar 2021)

2.2 Existing Stormwater Pipe Infrastructure

A review of the Dial Before You Dig (DBYD) information and survey prepared by SDG Land Development Solutions dated 14/04/2016 (refer to Appendix A and Figure 3). There is currently no stormwater drainage in either Holdsworth Ave and Berry Road. Currently property's fronting both of street discharge to kerb. The stormwater is currently collected by grated inlet pits at the end of both of these streets. Council is currently in the process of designing new stormwater drainage in both Holdsworth and Berry but the detailed inverts and locations are yet to be confirmed.

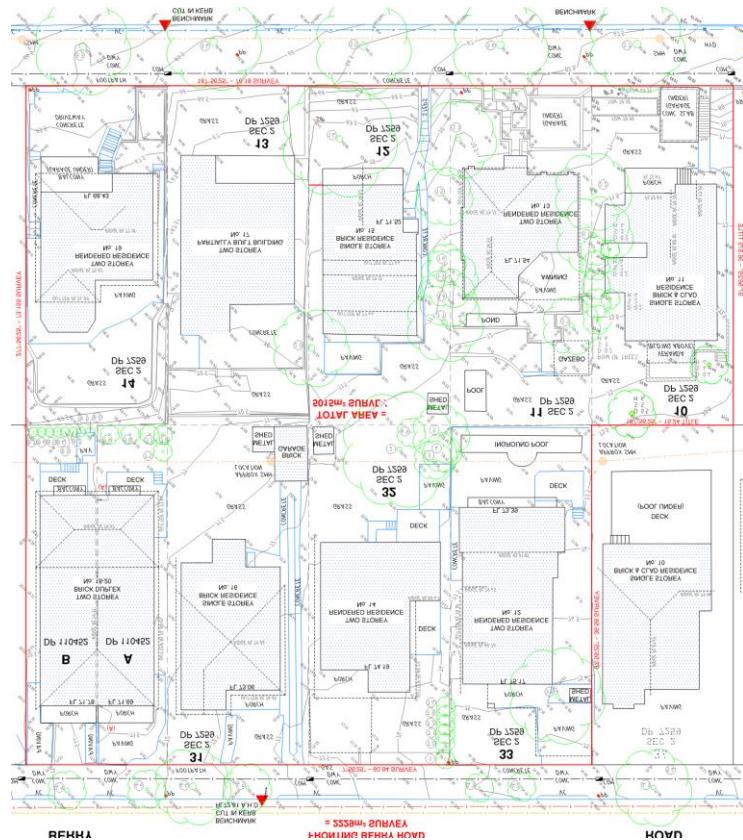


Figure 3: Survey Information by SDG Land Development Solutions

2.3 Existing Site Drainage Description

A survey of the site was completed by SDG Land Development Solutions dated 14/04/2016 but was not able to capture the internal drainage of the site due to the nature of the existing building. A review of the survey data shows the site slopes very steeply towards Holdsworth Street from Berry Road at an approximate slope of 8%. We make note of no existing easements shown on the survey to assist in the drainage of the existing lots.

3 PROPOSED DEVELOPMENT

The purposed development of the Site will consist of two multi-story buildings comprising private apartments, also known as flat residential buildings, 450m² childcare and 150m² community hall. Refer to Figure 4, which is indicated by the plans below.



Figure 4: Proposed Residential Flat Building – Ground Floor - (Silverster Fuller)

As a part of this development Aqualand will be constructing a portion of the proposed Pedestrian through site link as per the St Leonards South Landscape Master Plan dated October 2020.

Figure 5 below shows an overview of the plan and the section that will be dedicated to Council after construction. This portion has been treated separately from the proposed development with the through site link having a drainage system and WSUD infrastructure independent to the residential apartment.



Figure 5: St Leonards Landscaping Plan

4 DESIGN CRITERIA

4.1 Stormwater Policies and Objectives

An integrated stormwater management and water sensitive urban design concept has been prepared for the development. The strategy has been developed to meet the following objectives and comply with:

- Lane Cove Council DCP Part O: Stormwater Management
- Lane Cove Council DCP Part C: Residential Development

4.2 Proposed Drainage Design - Overview

On-Site Detention (OSD) is required as a part this development as outlined by the Lane Cove Council DCP Part O: Stormwater Management. Lane Cove Council requires new developments to comply with PSD (Permissible Site Discharge) and SSR (Site Storage Requirements) to restrict flow to pre-development conditions for all storms between 20% AEP (1 in 5 year) and 1% AEP (1 in 100 year). We have been advised by council that there will be a new, council owned, stormwater drainage pipe located under Holdsworth Avenue. We have designed in anticipation of this pipe line and may need to be reviewed once detailed information on the proposed line is acquired from council.

As per council recommendation, we will be discharging the lots containing the building independently from the lot dedicated to council's through site link. The residential building and its corresponding lots will drain to an OSD which will limit the discharge as per Lane Cove Council DCP Part O: Stormwater Management and improve the quality of the water discharged to be better than predevelopment quality using OceanGuards and stormfilter (discussed in a later section).

The through site link will discharge to the proposed stormwater pipe using an independent pit and pipe network. As per council advice, an OSD will not be provided for this lot but the quality of the water discharge from the site will be improved using OceanGuards (discussed in a later section).

4.3 Existing Development

As per the Council requirement the existing condition is required to determine the OSD requirements and the WSUD requirements. Below, figure 6, is an image of the pre-existing catchment plan used to calculate the impervious area in the pre-developed state.

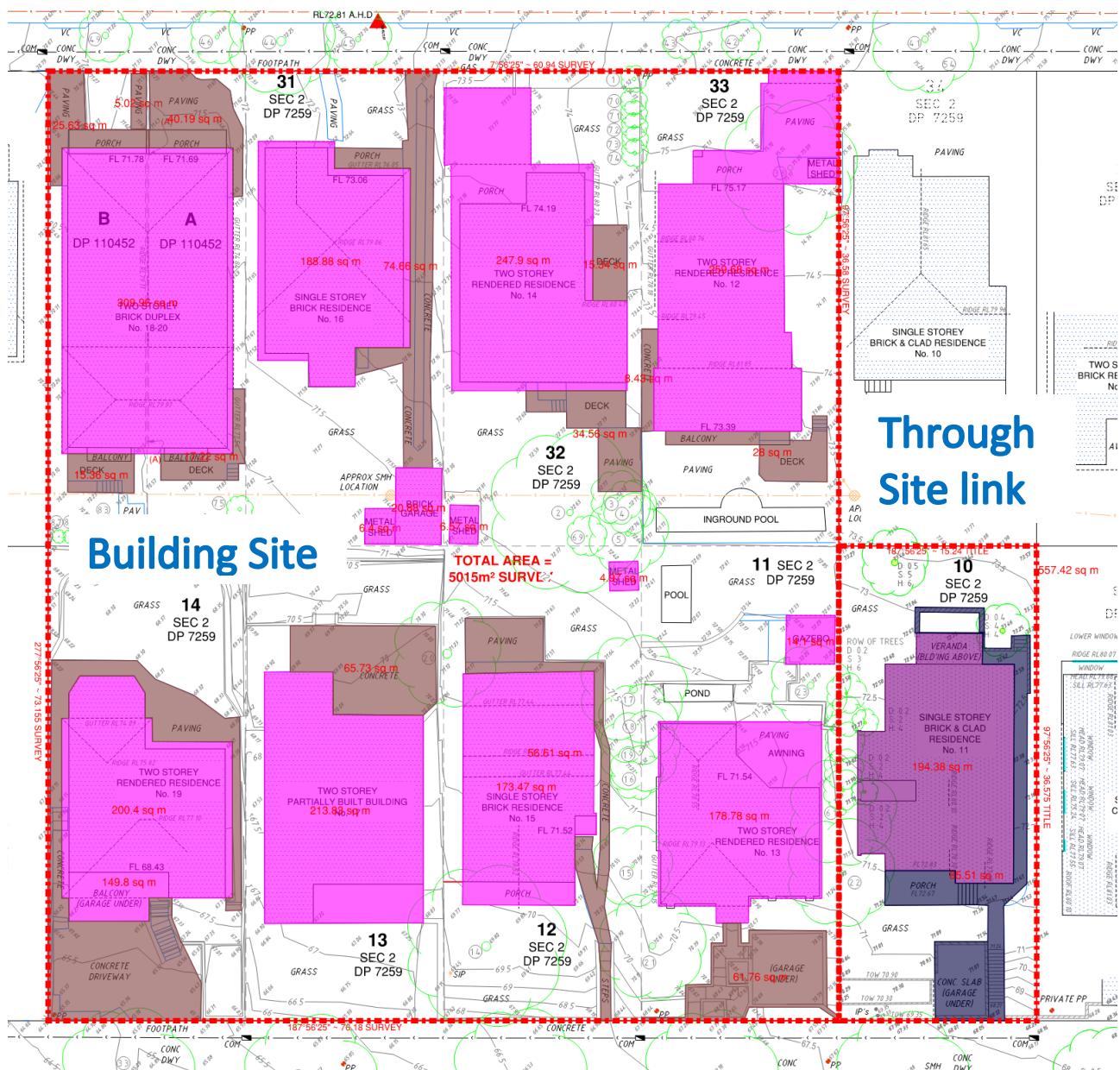


Figure 6: Pre-Development Catchment

Table 2: Summary of pre-development catchment plan areas

Building Lots	m^2		Through Site Link	m^2
Roof	1826		Roof	195
Pervious Landscaping	599		Pervious Landscaping	276
Paved	2034		Paved	85.5

Due to the nature of the Site there us a portion of area that is not able to drain to the OSD Tank via Gravity. This requires the PSD from the OSD tank to be adjusted to make sure the overall Site PSD is complaint. To calculate the bypassing PSD rate the rational method was used.

Below is a summarised table of areas in the post developed state;

Building Lots	m^2		Through Site Link	m^2
Roof	1629		Roof	0
Landscaping (Treated)	1349		Landscaping (Treated)	157.5
Paved (Treated)	1215.6		Paved	400
Driveway	26			
Landscape (Untreated)	62.4			
Paved (Untreated)	176			

4.4 Proposed Drainage Design – Building

Council requires in there DCP Part O Stormwater Management states that permissible Site Discharge (PSD) and Site Storage Requirement (SSR) is based on per Site area.

PSD: 140 l/s/ha

SSR: $0.0255m^3/m^2$

The PSD and SSR where calculated based on Council's On-Site Detention Calculations Sheet refer to Appendix C.

The results of the Council's On-Site Detention Calculations sheet were as follows:

PSD = 54.54 L/s

SSR = $112.14 m^2$

4.5 Proposed Drainage Design – Through Site Link

As per council's recommendation we have not provided an OSD for this site as the flow does not need to be restricted and can be allowed to flow fully to council systems. We have proposed to install a pit and pipe system to collect the water from the site to limit uncontrolled overland flow and discharge this water to councils proposed stormwater system in a controlled manner.

4.6 Water sensitive Urban Design

The main objectives for stormwater quality are indicated in Lane Cove Council DCP Part O: Stormwater Management. The design is required to improve the quality of the discharge from the site from its pre-developed state. Methods to achieve the above treatment targets can include the use of proprietary products, rainwater reuse, raingardens, detention basins or treatment further downstream within the external stormwater network.

5 ANALYSIS RESULTS

5.1 On-Site Detention Design

Stormwater runoff from the development is required to be discharged at a controlled rate. The design is governed by the requirement set out in Lane Cove Council DCP Part O: Stormwater Management

5.2 Water Sensitive Urban Design Implementation

The water quality modelling software MUSIC v6.3 was used to analyse the performance of the treatment train. Figure 6 below shows the MUSIC node and link diagram used to describe the proposed treatment train. The model has been built to assess the adequacy of the proposed stormwater treatment measure and ensure that stormwater quality meets the objectives prior to stormwater runoff leaving the Site.

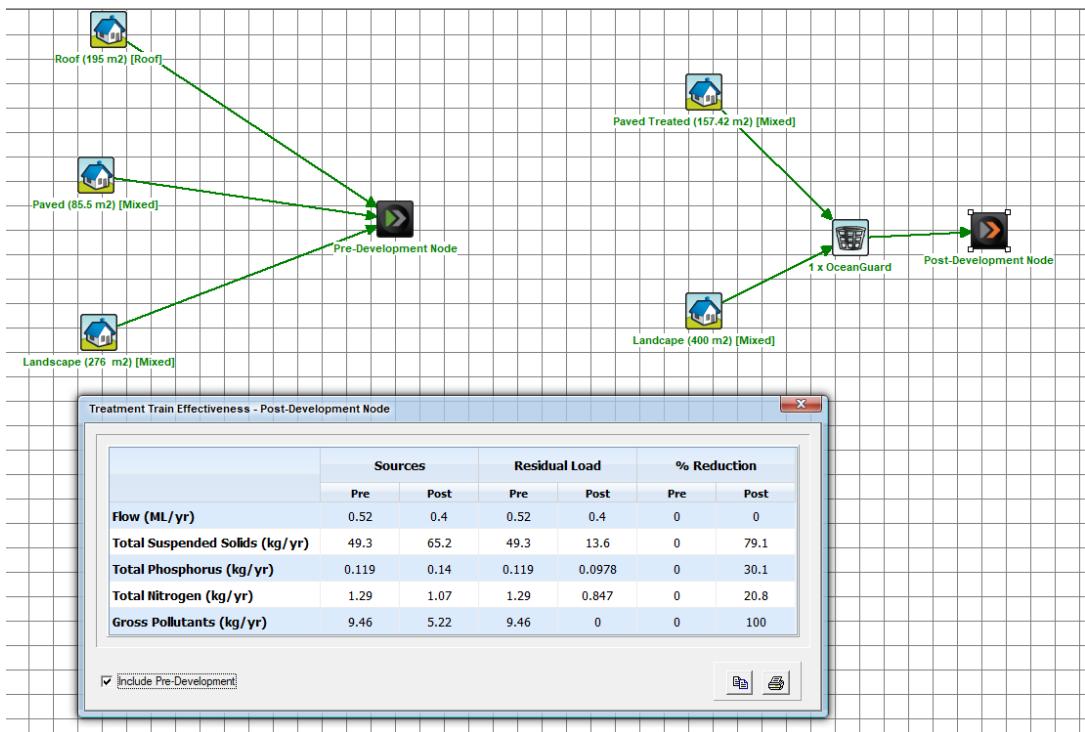


Figure 7: Music Model and Results – Through Site Link

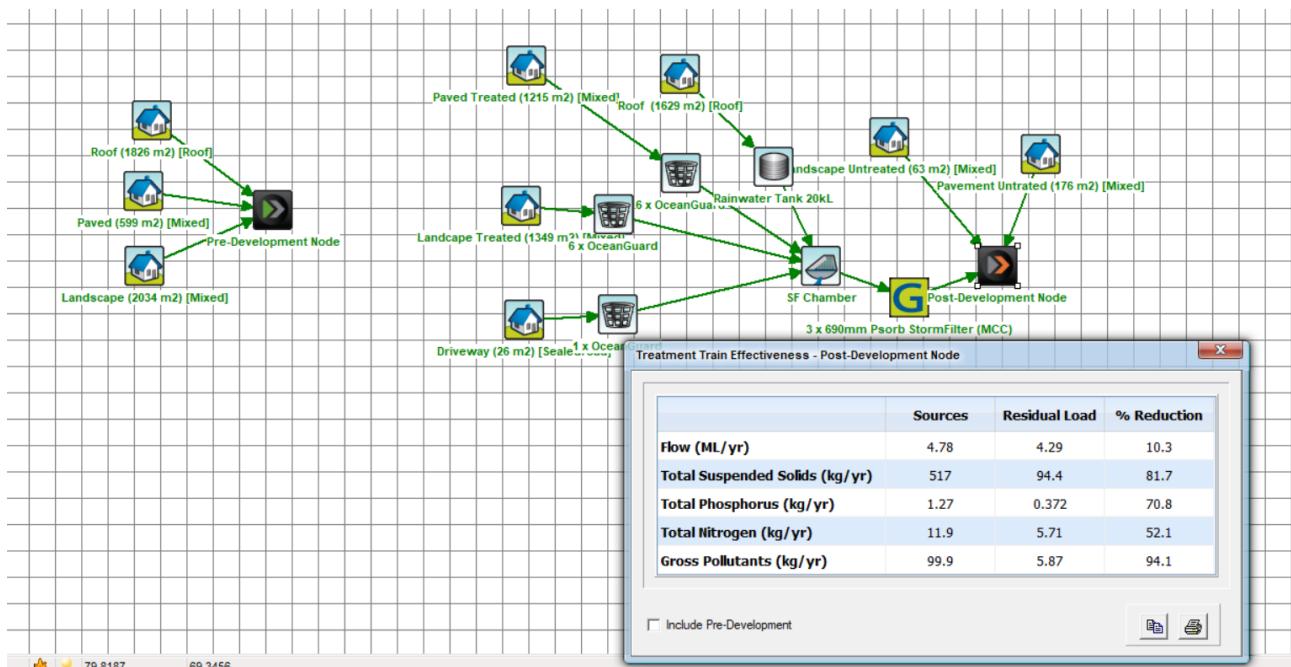


Figure 8: Music Model and Results - Building Only

The results of the analysis showed the treatment train would achieve the water quality targets set out in Council's DCP. Table 3 and 4 below displays the effectiveness of the treatment train for the primary and secondary treatment. To meet the requirements set out by Council, the following treatment devices were implemented 3 x 690 Psorb (MCC) StormFilter, 13 OceanGuard from Ocean Protect in the building lots and 1 x OceanGuard in the through site link.

The water quality model created using MUSIC software provides an indication of the pollutant removal rates expected when a treatment train of water quality measures is applied to the proposed layout of the development.

Table 3: MUSIC Model Results – Through Site link

Pollutant	Pre-Development Loads (kg/yr)	Post Development Loads (kg/yr)	Water Quality Objective (Pre < Post)	Percentage Reduction Achieved (%)
Gross Pollutants (GP)	9.46	0	YES	100
Total Suspended Solids (TSS)	49.3	13.6	YES	79.1
Total Phosphorus (TP)	0.119	0.0978	YES	30.1
Total Nitrogen (TN)	1.29	0.847	YES	20.8

Table 3: MUSIC Model Results – Building Only

Pollutant	Pre-Development Loads (kg/yr)	Post Development Loads (kg/yr)	Water Quality Objective (Pre < Post)	Percentage Reduction Achieved (%)
Gross Pollutants (GP)	79.3	5.87	YES	94.1
Total Suspended Solids (TSS)	258	93.7	YES	80.4
Total Phosphorus (TP)	0.913	0.377	YES	70.1
Total Nitrogen (TN)	9.3	6.07	YES	48.6

5.3 Proposed Stormwater Treatment Train

5.3.1 Ocean Protect 690 PSORB Stormfilter Cartridges (MCC)

A total of three (3) cartridges are to be introduced to the design as a major filter device located within the WSUD tank at the end of the treatment train. The capture rates provided by the manufacturer, Stormwater360:

- TSS 73.5%
- TN 32%
- TP 49%
- Litter 95%

5.3.2 Ocean Protect OceanGuard

A total of one (1) OceanGuard inserts will be used as a pre-treatment for stormwater runoff to capture litter and coarse sediment surface flows on the Site. OceanGuard inserts are to be installed on all surface inlet pits across the Site. The following capture rates have been adopted for the MUSIC model, based on information provided by Stormwater360:

- TSS 54%
- TN 21%
- TP 30%
- Litter 100%

5.3.3 20kL Rainwater Tank

A 20kL rainwater tank will be implemented to capture stormwater runoff generated off the roof of the residential flat buildings. The collected rainwater will be used for irrigation of the landscaped areas across the Site.

6 CONCLUSION

BG&E has been engaged by Aqualand to prepare a Stormwater Management Report for the proposed development of residential apartment buildings at 12-20 Berry Road & 11-19 Holdsworth Avenue St Leonards South NSW 2065 Australia

This report has outlined the methodology adopted and associated results of:

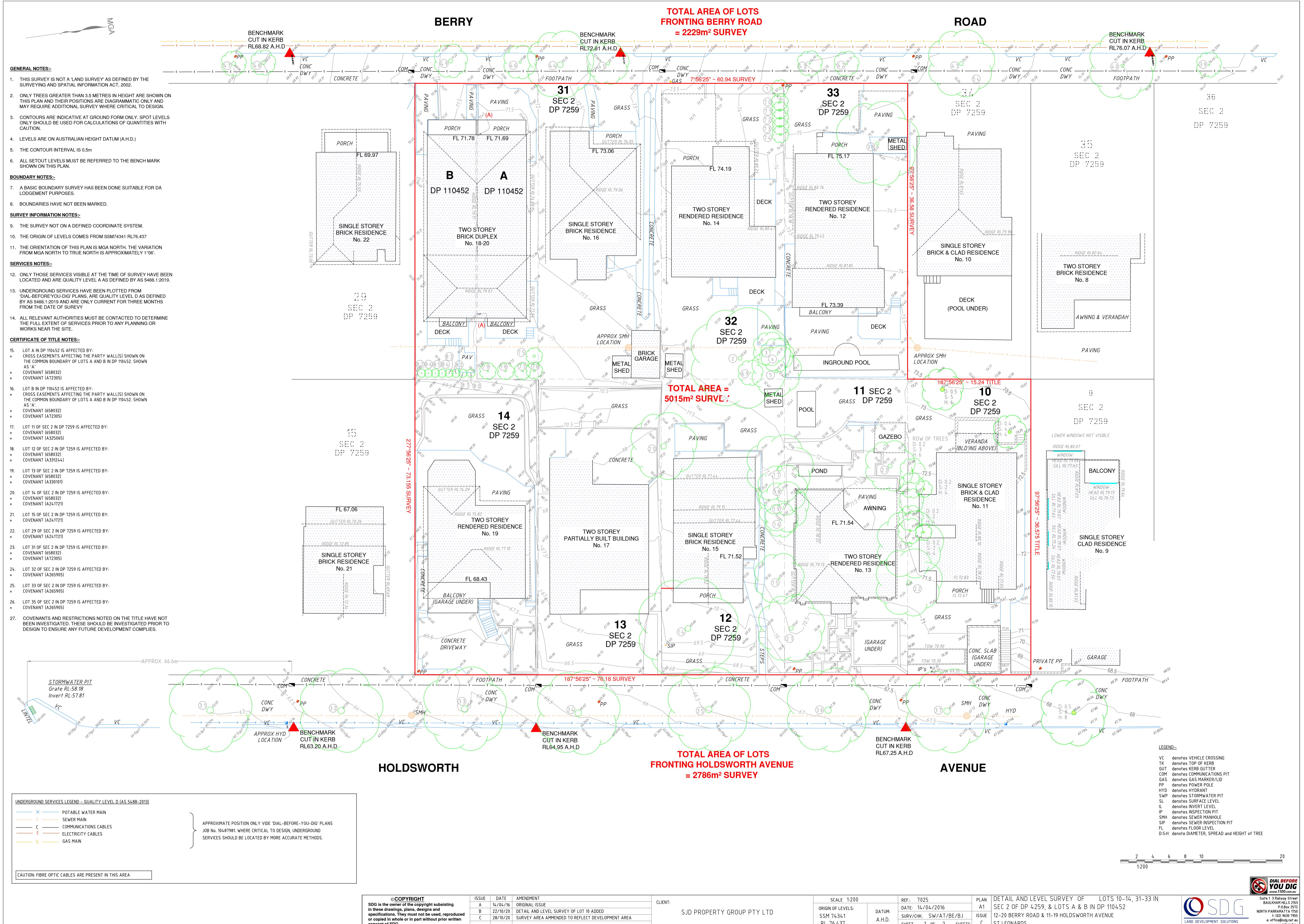
- Requirement of a detention tank; and
- Water sensitive urban design.

As outlined in this report, the following key items have been identified:

- The Site previously was connected to the road, discharging all water from the separate lots to the kerb;
- The proposed stormwater discharge line from the Site has been design to allow for a future connection into Council proposed stormwater line to run along Holdsworth Ave;
- A detention tank is required and will provide 123.4m³ of storage; and
- The building site is expected to discharge approximately 54.54 L/s from the Site during a 1 in 100 Year ARI Storm Event;
- The discharge from the through site link has not been controlled as per council's suggestion but the water quality has been treated to be better than pre-development conditions;
- Water Sensitive Urban Design is required as part of the development of the building site and through site link. This has been achieved using proprietary products. The building site requires 3 x 690 Psorb(MCC) StormFilter and 13 OceanGuard from Ocean Protect in the building space and 3 x OceanGuards separately in the through site link.

APPENDIX A

Site Survey



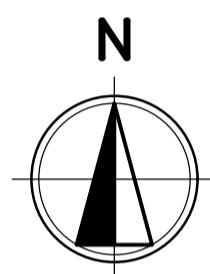
APPENDIX B

Stormwater Plan

12-20 BERRY RD & 11-19 HOLDSWORTH AVENUE, ST LEONARDS SOUTH NSW 2065, AUSTRALIA

LGA: NORTH SYDNEY COUNCIL
DEVELOPMENT APPLICATION ISSUE

**BG
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DRAWING INDEX

DRAWING No.	SHEET DESCRIPTION
CI-0000	COVER SHEET, LOCALITY PLAN AND DRAWING INDEX
CI-0200	SITWORKS AND DRAINAGE BASEMENT 1
CI-0201	SITWORKS AND DRAINAGE GROUND FLOOR
CI-0270	DRIVeway SECTIONS
CI-0300	DRAINAGE CATCHMENT PLAN
CI-0340	DRAINAGE DETAILS
CI-0350	OSD PLAN
CI-0351	OSD SECTIONS AND DETAILS
CI-0700	EROSION AND SEDIMENT CONTROL PLAN
CI-0710	EROSION AND SEDIMENT CONTROL DETAILS



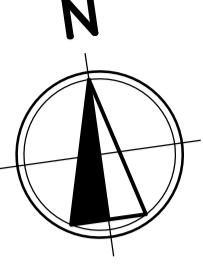
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SCALE 1:1000



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SCALE 1:1000 AT A1 SIZE

NSW

CLIENT										ARCHITECT				PROJECT				STATUS				ISSUED FOR APPROVAL				TITLE			
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REV	DATE	DESCRIPTION	REV	REVISIONS	REV	REVISIONS	REV	REVISIONS																					
P-1805LS202206100 DRAWN002.EVIL/AUTODRAGS2022-DRG-CI-0000.DWG 9/06/2022 4:07:21 PM																													



LEGEND

SITE BOUNDARY

PROPOSED STORMWATER (SIZE AND TYPE)

PROPOSED GRATED INLET/JUNCTION PIT

PROPOSED GRATED DRAIN

GD

ARCHITECTURAL

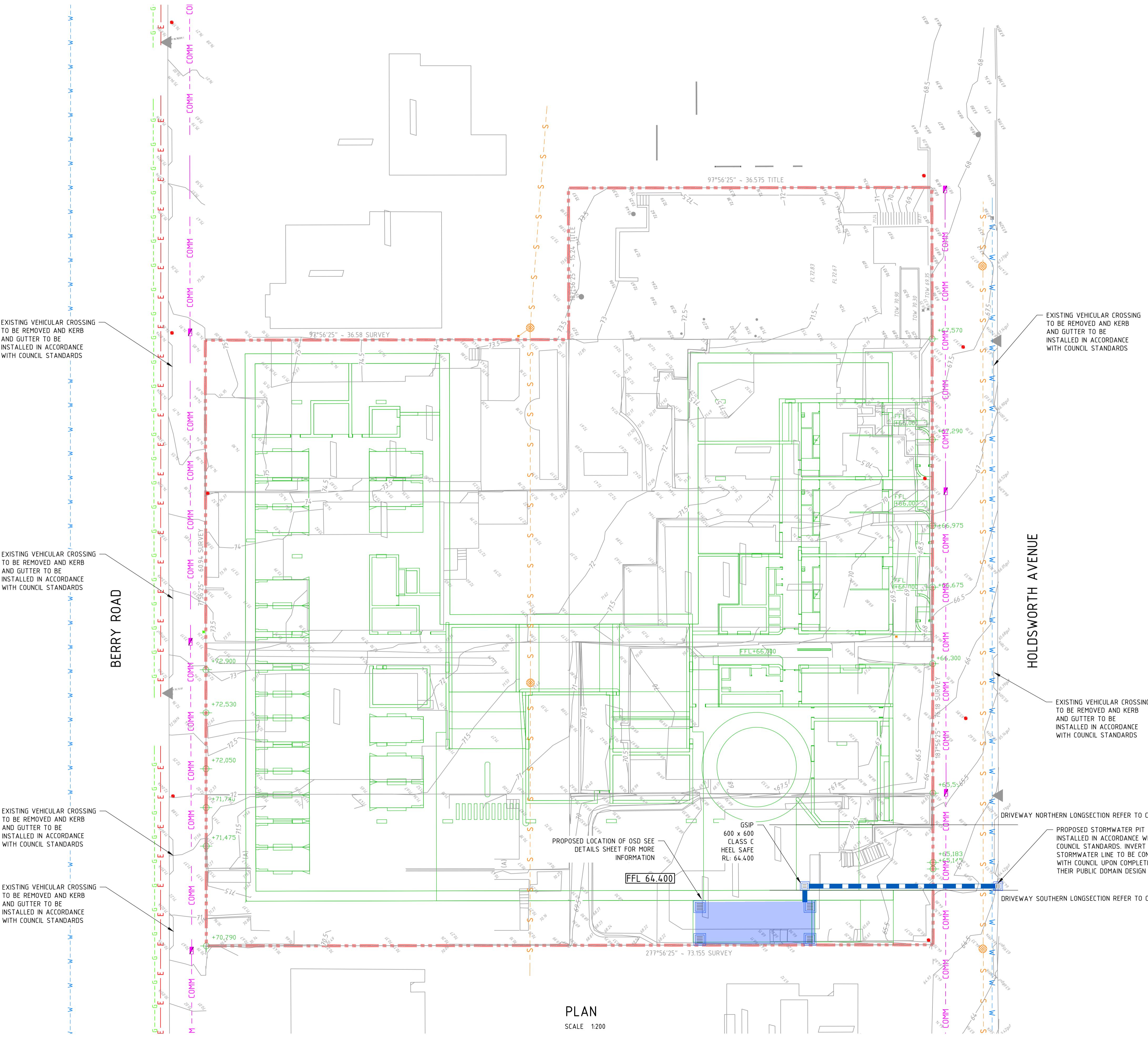
NEIGHBOURING DEVELOPMENT

EXISTING SERVICES

— E — E —	EXISTING ELECTRICITY UNDERGROUND EXISTING ELECTRICITY
— OH —	EXISTING ELECTRICITY OVERHEAD
— S — S —	EXISTING SEWER
— COMM —	EXISTING COMMS
— W — W —	EXISTING WATER
— G — G —	EXISTING GAS
— SW —	EXISTING STORMWATER

NOTES

1. OCEAN PROTECT OCEANGUARD OR EQUIVALENT PRODUCT TO BE INSTALLED TO ALL GRATED INLET PITS.
 2. ALL GRATED TRENCH DRAINS TO BE 100(D)x100(W) EXCEPT FOR THE DRIVEWAY GRATED TRENCH DRAINS TO BE 200(W)x200(D).



SCALE 1:2



SILVESTER RÉLL

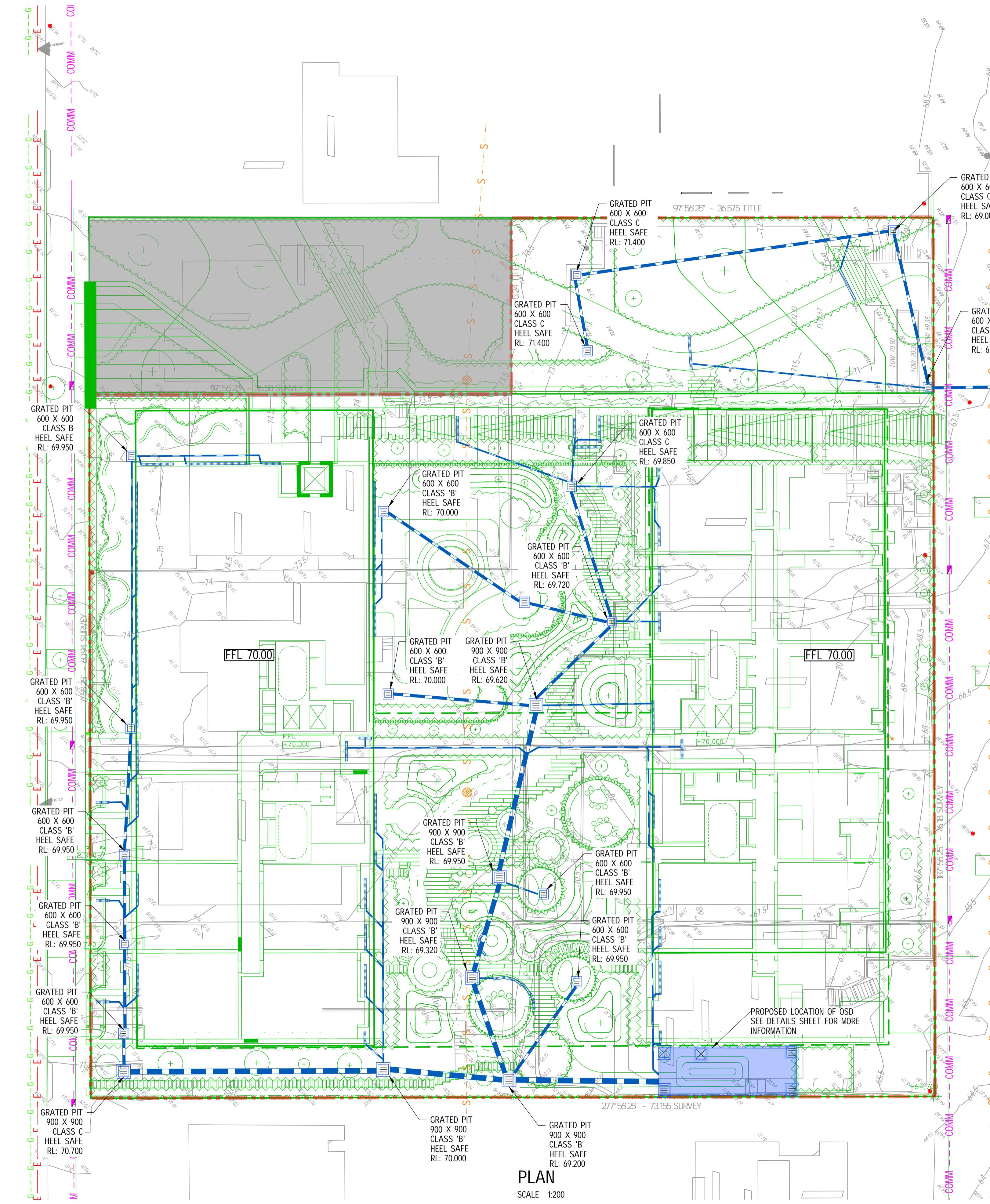
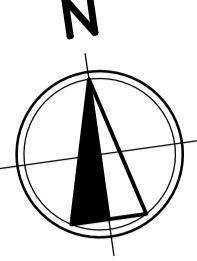
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BG
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12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA

<h1 style="margin: 0;">ISSUED FOR APPROVAL</h1> <p style="margin: 0;">NOT TO BE USED FOR CONSTRUCTION</p>			
DRAWN	DESIGNED	CHECKED	APPROVED
FM	AM	SH	
DATUM	GRID GDA2020	SCALE	

SITEWORKS AND DRAINAGE BASEMENT 1

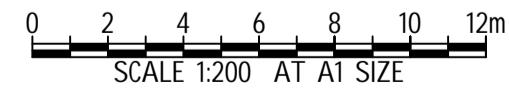


EXISTING SERVICES

— E — E —	EXISTING ELECTRICITY UNDERGROUND
— OH —	EXISTING ELECTRICITY OVERHEAD
— S — S —	EXISTING SEWER
— COMM —	EXISTING COMMS
— W — W —	EXISTING WATER
— G — G —	EXISTING GAS
— SW —	EXISTING STORMWATER

1. OCEAN PROTECT OCEANGUARD OR EQUIVALENT PRODUCT TO BE INSTALLED TO ALL GRATED INLET PITS.
 2. ALL GRATED TRENCH DRAINS TO BE 100(D)x100(W) EXCEPT FOR THE DRIVEWAY GRATED TRENCH DRAINS TO BE 200(W)x200(D).

36.05gut
18/12/59



D	16.06.22	ISSUED FOR DEVELOPMENT APPROVAL
C	14.06.22	ISSUED FOR DEVELOPMENT APPROVAL
B	14.06.22	ISSUED FOR DEVELOPMENT APPROVAL
A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL
REV	DATE	DESCRIPTION



SILVESTER ELLER

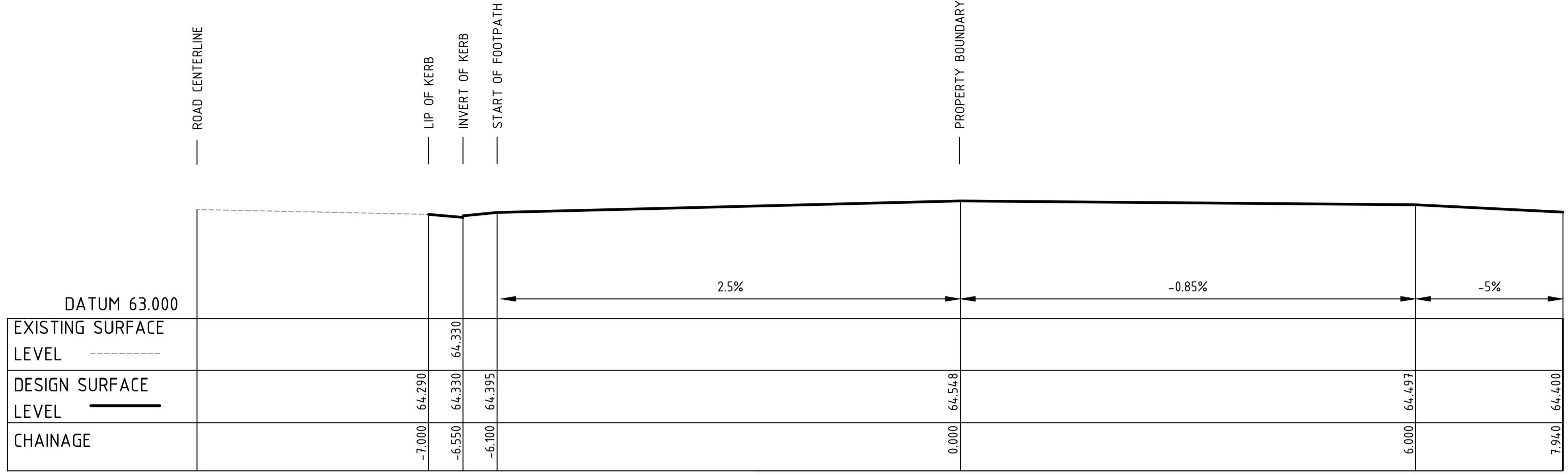
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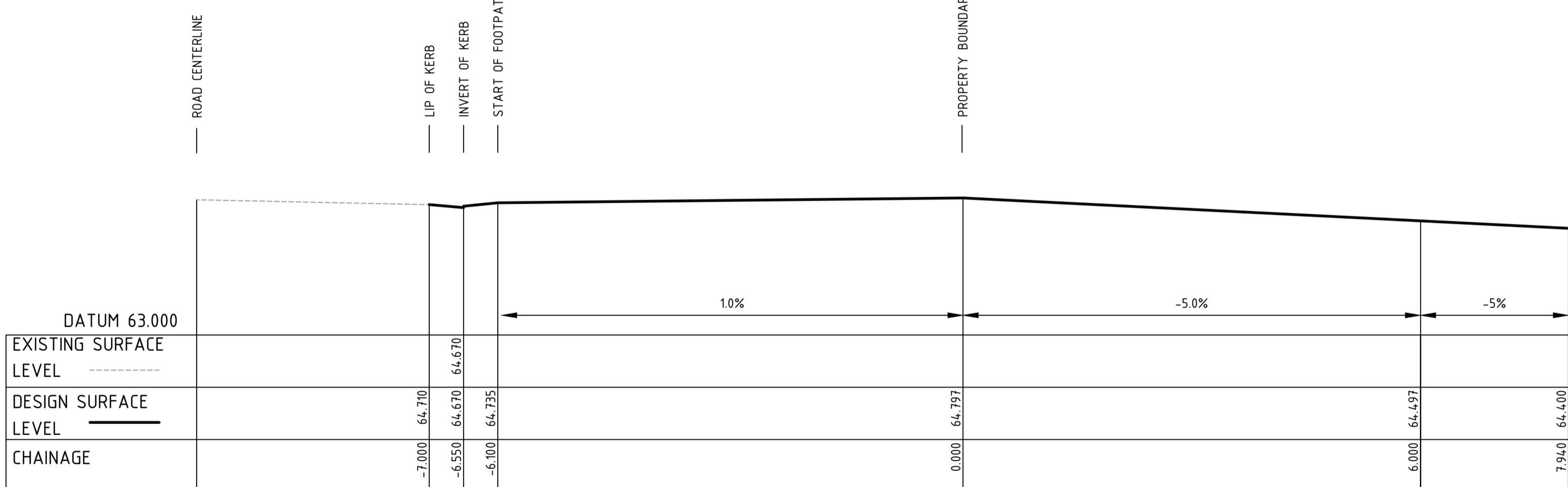
**12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA**

ISSUED FOR APPROVAL			
NOT TO BE USED FOR CONSTRUCTION			
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DATUM AHD	GRID GDA2020 MGA 56	SCALE 1:200	AT

SITEWORKS AND DRAINAGE GROUND FLOOR



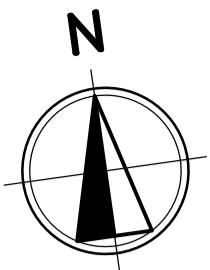
DRIVEWAY SOUTHERN LONGITUDINAL SECTION



DRIVEWAY NORTHERN LONGITUDINAL SECTION

0 0.5 1.0 1.5 2.0 2.5 3.0m
SCALE 1:50 AT A1 SIZE

CLIENT										ARCHITECT			PROJECT			STATUS				TITLE		
AQUALAND										SILVESTER FULLER			Sydney Office – L2, 8 Windmill St Sydney NSW 2000 P / +61 2 9770 3300 E / info@bgeeng.com bgeeng.com –			ISSUED FOR APPROVAL NOT TO BE USED FOR CONSTRUCTION				DRIVEWAY SECTIONS		
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REV		DESCRIPTION													DATUM	GRID	SCALE			PROJECT No.	DRAWING No.	REV.
		REVISIONS													AHD	GDA2020	MGA-56	AS SHOWN	AT A1 SIZE	S21268	CI-0270	A



HATCHED AREA NOT PART OF THIS DA APPLICATION

BERRY ROAD

HOLDSWORTH AVENUE

WATER QUALITY FOR DEVELOPMENT

4458 m²
TREATMENT DEVICES:
- WATER QUALITY CHAMBER WITH 3 OCEAN PROTECT 690 PSORB (MCC) STORMFILTER CARTRIDGES
- 13 X OCEANGUARD

TREATMENT STANDARDS

POLLUTANT	PRE	POST	COUNCIL REQUIREMENT REDUCTION FROM PRE TO POST BY:
GROSS POLLUTANTS	79.3	5.87	OK
TOTAL SUSPENDED SOLIDS	258	93.7	OK
TOTAL PHOSPHORUS	0.913	0.377	OK
TOTAL NITROGEN	9.3	6.07	40

STORMWATER DRAINAGE REQUIREMENTS HAVE BEEN CALCULATED IN ACCORDANCE WITH LANE COVE COUNCIL DCP SECTION 0: STORMWATER MANAGEMENT

WATER QUALITY FOR THE THROUGH SITE LINK

557.4 m²
TREATMENT DEVICES:
- 1 x OCEANGUARD

TREATMENT STANDARDS

POLLUTANT	PRE	POST	COUNCIL REQUIREMENT REDUCTION FROM PRE TO POST BY:
GROSS POLLUTANTS	9.46	0	OK
TOTAL SUSPENDED SOLIDS	49.3	13.6	OK
TOTAL PHOSPHORUS	0.119	0.0978	OK
TOTAL NITROGEN	1.29	0.847	OK

STORMWATER DRAINAGE REQUIREMENTS HAVE BEEN CALCULATED IN ACCORDANCE WITH LANE COVE COUNCIL DCP SECTION 0: STORMWATER MANAGEMENT

SCALE 1:200 AT A1 SIZE

LEGEND

ROOF CATCHMENT 1629 m ²
PAVED AREA (UNTREATED) CATCHMENT 176 m ²
PAVED AREA (TREATED) CATCHMENT 1215 m ²
PAVED AREA (PARK) CATCHMENT 157 m ²
LANDSCAPE (TREATED) CATCHMENT 1349 m ²
LANDSCAPE (UNTREATED) CATCHMENT 63 m ²
LANDSCAPE (PARK) CATCHMENT 400 m ²
DRIVEWAY CATCHMENT 26 m ²

REVISIONS			
B	16.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH
A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH
REV	DATE	DESCRIPTION	RVD



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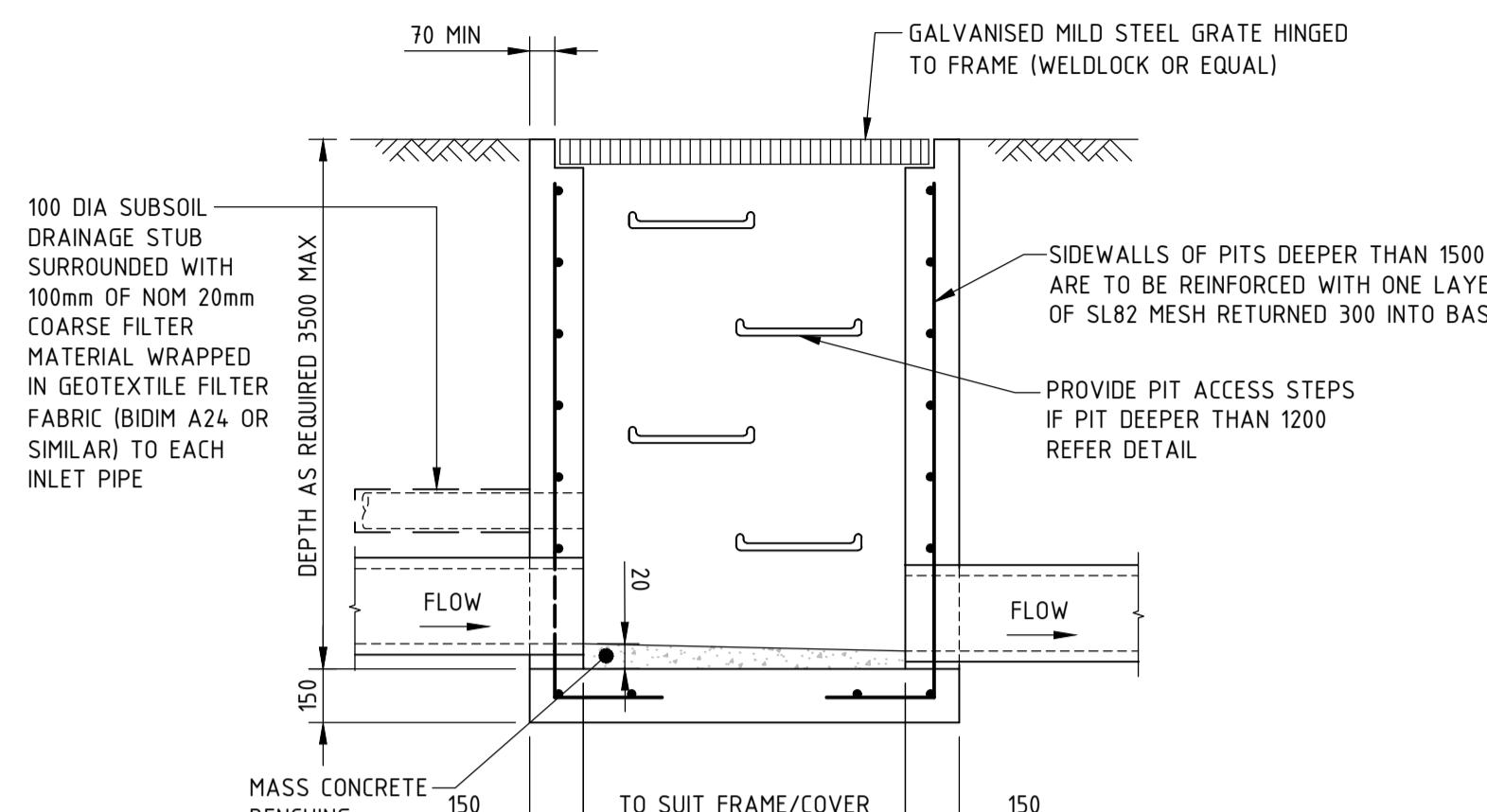
BG&E
PROJECT
12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA

STATUS
ISSUED FOR APPROVAL
NOT TO BE USED FOR CONSTRUCTION
DRAWN
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CHECKED
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APPROVED

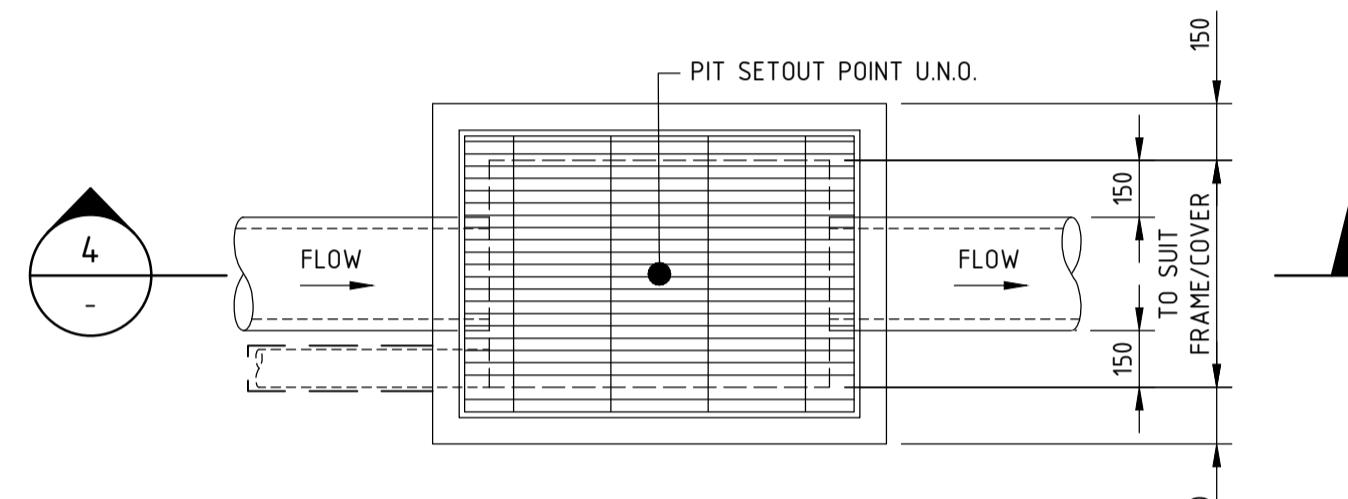
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S21268
DRAWING NO.
CI-0300
REV.
B

NOTES

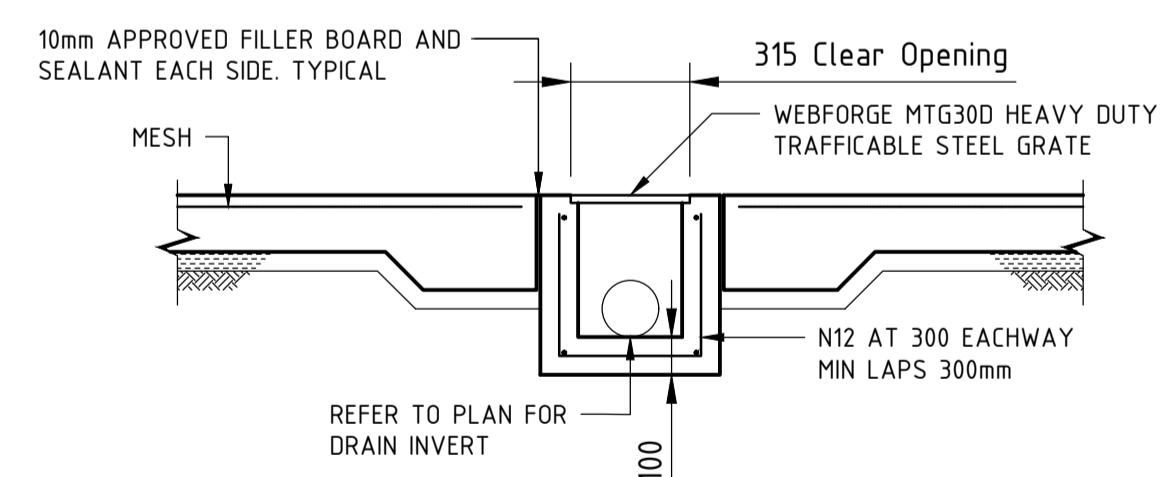
1. ALL DIMENSIONS ARE IN MILLIMETRES U.N.O.



SECTION 4
SCALE 1:20



PLAN - GRATED / JUNCTION PIT DETAIL
SCALE 1:20



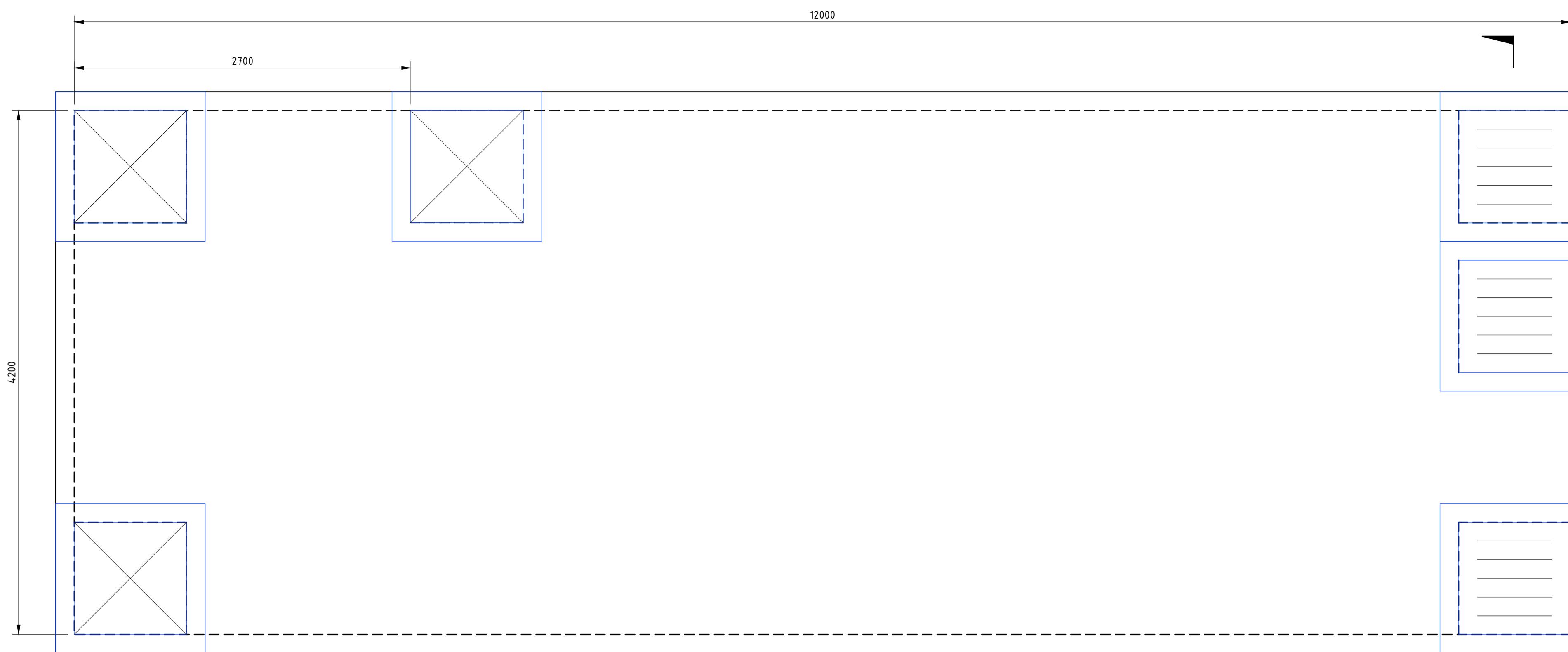
GRATED TRENCH DETAIL
SCALE : N.T.S

0 0.2 0.4 0.6 0.8 1.0 1.2m 0 100 200 300 400 500 600mm
SCALE 1:20 AT A1 SIZE SCALE 1:10 AT A1 SIZE

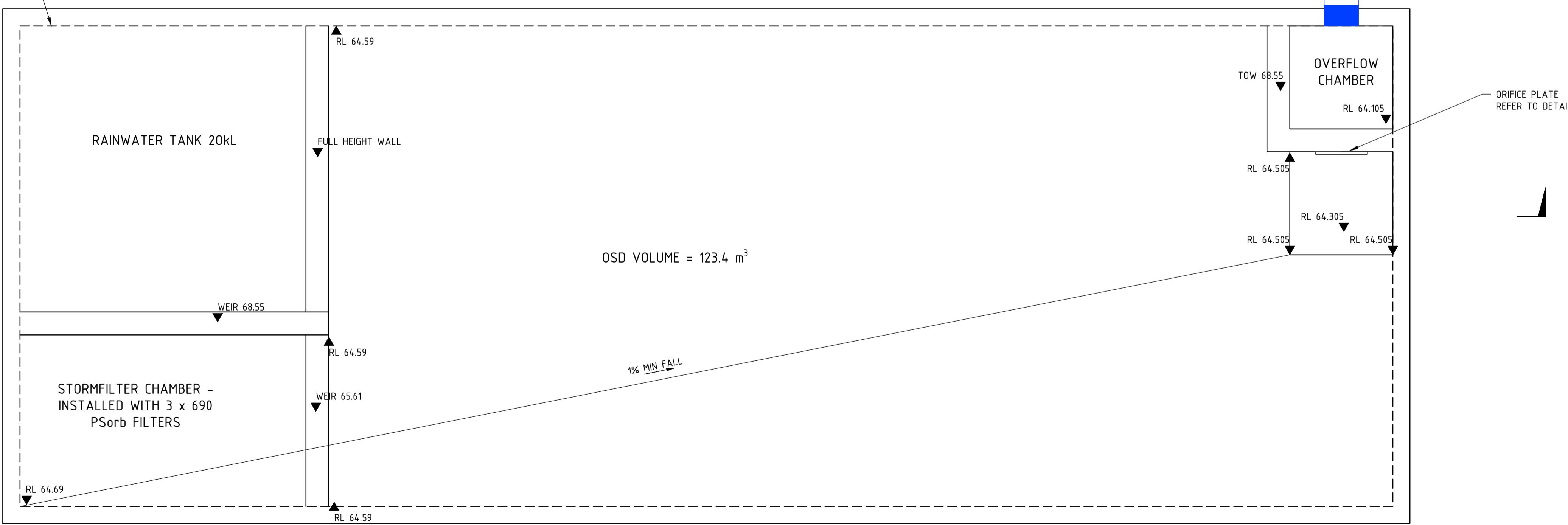
										CLIENT				ARCHITECT			PROJECT			STATUS						
																	Sydney Office – L2, 8 Windmill St Sydney NSW 2000 P/ +61 2 9770 3300 E/ info@bgeeng.com bgeeng.com-			ISSUED FOR APPROVAL NOT TO BE USED FOR CONSTRUCTION				DRAINAGE DETAILS		
A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL		SH	RVD	REV	DATE	DESCRIPTION	RVD					F.M	D.GRN	CHECKED	APPROVED	PROJECT No.	DRAWING No.	REV.						
REV	DATE	DESCRIPTION	REVISIONS	SH	RVD	REV	DATE	DESCRIPTION	RVD					AHD	GDA2020 MGA-56	SCALE	AS SHOWN	AT A1 SIZE	AT A1 SIZE	REV.						
			REVISIONS					REVISIONS						BG & E	12-20 BERRY ROAD & 11-19 HOLDWORTH AVENUE ST LEONARDS SOUTH NEW 2065 AUSTRALIA			PROJECT No.	DRAWING No.	REV.						
P:\\BGE\\SYD\\S2726\\100 DRAW\\100_E\\CIVIL\\AUTOCAD\\S2726-DRG-C-0340.DWG 9/06/2022 4:12:14 PM																		S21268	CI-0340	A						

NOTES

1. ALL DIMENSIONS ARE IN MILLIMETRES U.N.O.

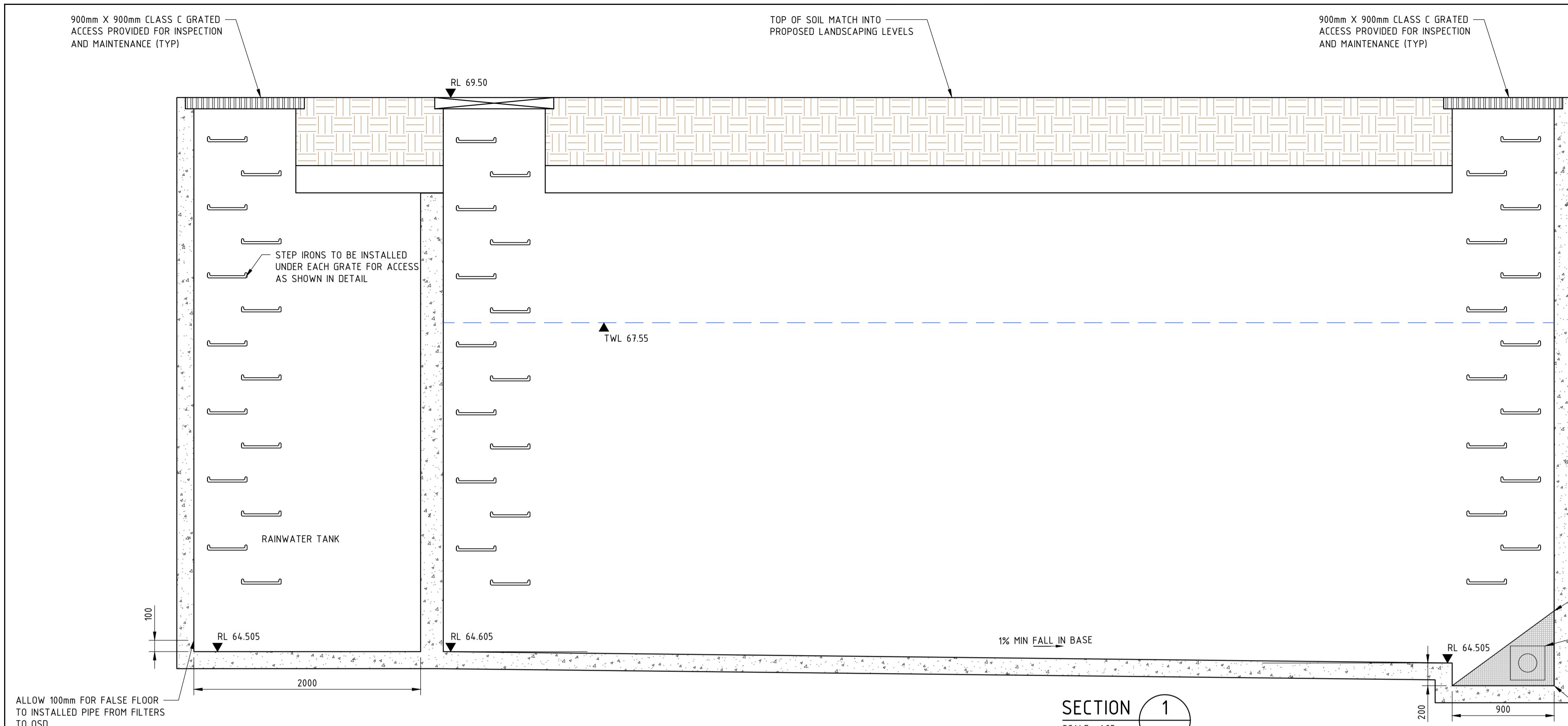


100mm FALSE FLOOR TO BE
INSTALLED TO ALLOW FOR
INSTALLATION OF TREATED LINE
FROM STORMFILTERS



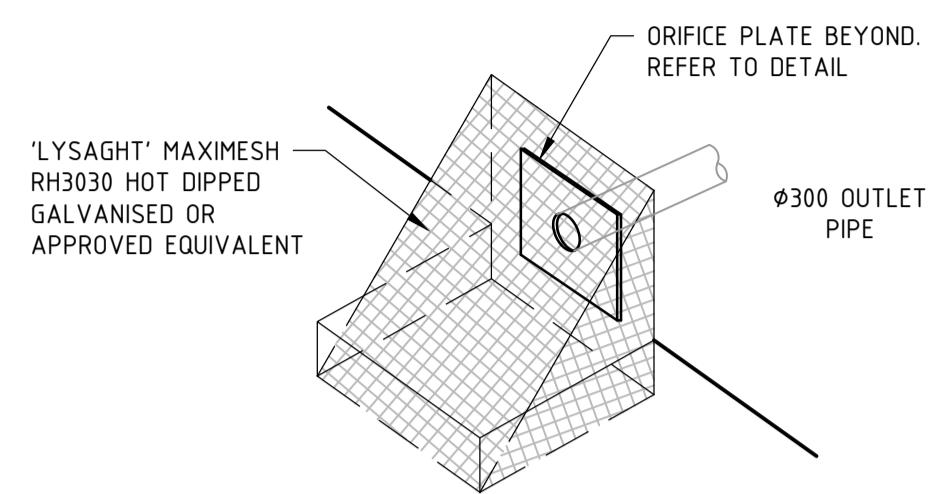
0 0.25 0.5 0.75 1.0 1.25 1.5m
SCALE 1:25 AT A1 SIZE

CLIENT										ARCHITECT			PROJECT			STATUS				TITLE	
B	14.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH	SILVESTER FULLER	E	AQUALAND	Sydney Office –	12-20 BERRY ROAD &	12-20 BERRY ROAD &	ISSUED FOR APPROVAL	NOT TO BE USED FOR CONSTRUCTION			OSD PLAN							
A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH	RVD	REV	DATE	DESCRIPTION	11-19 HOLDWORTH AVENUE	11-19 HOLDWORTH AVENUE	DRAWN	DESIGNED	CHECKED	APPROVED								
REV		DESCRIPTION		RVD	REV	DATE	DESCRIPTION	RVD	RVD	FM	AM	SH									
		REVISIONS					REVISIONS			AHD	GDA2020	SCALE		PROJECT No.	DRAWING No.	REV					
		REVISIONS					REVISIONS			AHD	MGA-56	AS SHOWN		S21268	CI-0350	B					
P180617SY01S21268.DWG	14/06/2022	17:39:51 AM																			



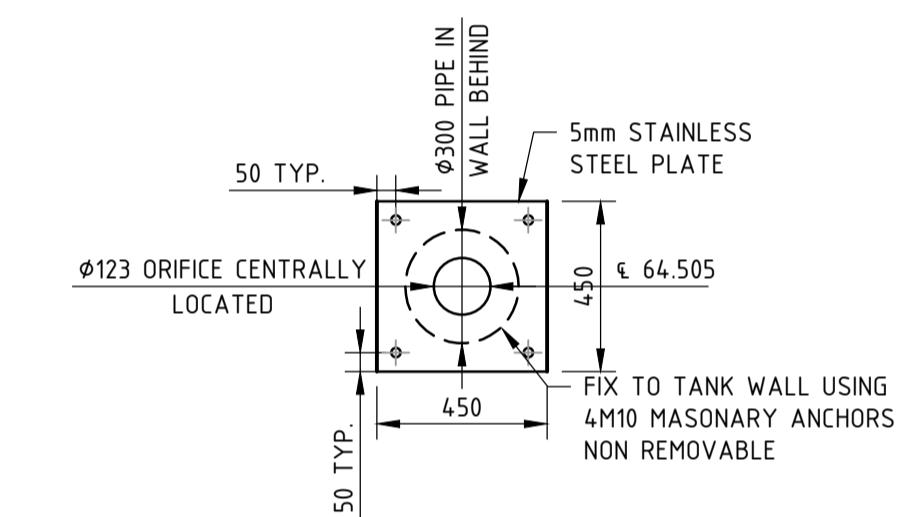
NOTES

1. ALL DIMENSIONS ARE IN MILLIMETRES U.N.O.



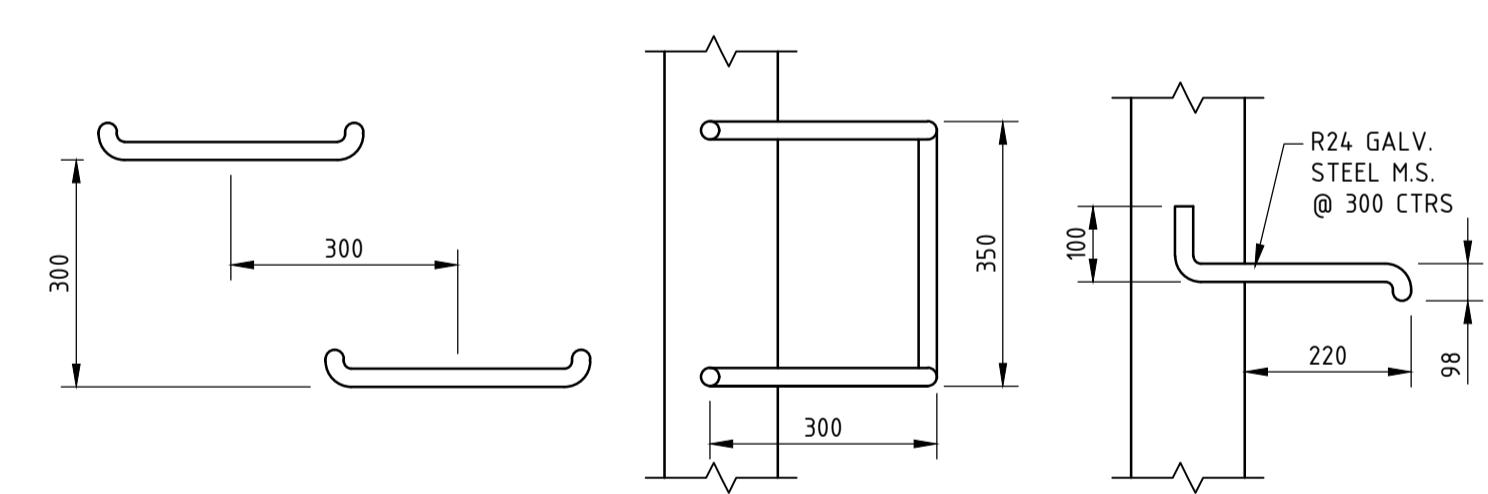
TRASH SCREEN DETAIL

N.T.S

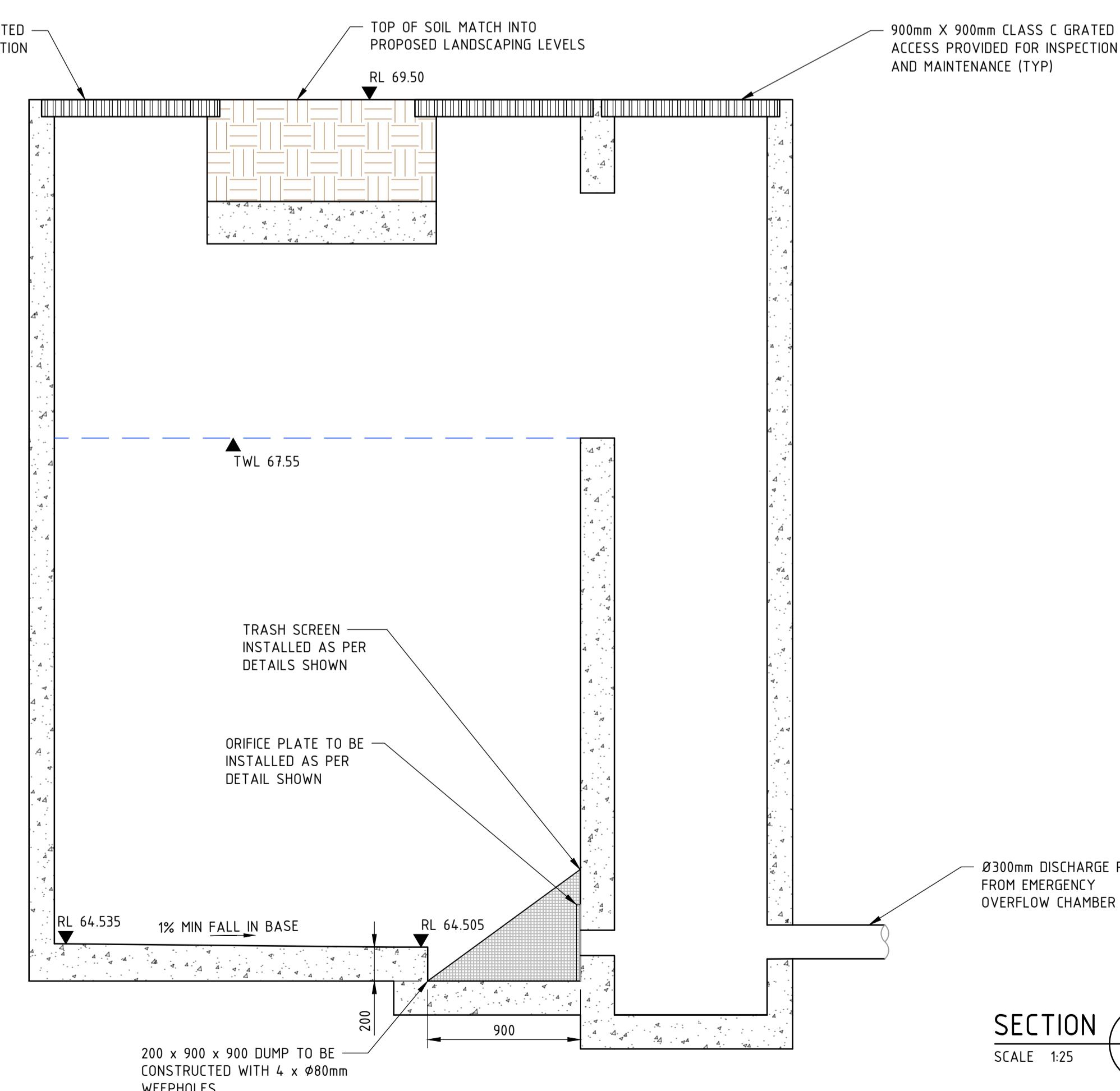


Ø150 ORIFICE PLATE DETAIL

SCALE 1:20



TYPICAL STEP IRON DETAILS



900mm X 900mm CLASS C GRATED
ACCESS PROVIDED FOR INSPECTION
AND MAINTENANCE (TYP)

ALLOW 100mm FOR FALSE FLOOR –
TO INSTALLED PIPE FROM FILTERS
TO OSD

900mm X 900mm CLASS C GR
ACCESS PROVIDED FOR INSPE
AND MAINTENANCE (TYP)

TOP OF SOIL MATCH INTO
PROPOSED LANDSCAPING L

900mm X 900mm CLASS C GRA
ACCESS PROVIDED FOR INSPEC
AND MAINTENANCE (TYP)

- TRASH SCREEN
INSTALLED AS PER
DETAILS SHOWN
- ORIFICE PLATE TO BE
INSTALLED AS PER
DETAIL SHOWN
- 200 x 900 x 900 DUMP TO BE
CONSTRUCTED WITH 4 x Ø80mm
WEEPHOLES

The diagram shows a cross-section of a flow measurement assembly. It consists of a central vertical pipe with an outer diameter of $\phi 300$ mm. A $\phi 123$ mm orifice plate is centrally located on the pipe. The pipe is supported by a 5mm stainless steel plate attached to the tank wall behind it. The distance from the bottom of the pipe to the top of the support plate is 450 mm. The distance from the centerline of the pipe to the centerline of the orifice plate is 50 mm (TYPICAL). The total height of the assembly is 64.505 mm. The assembly is fixed to the tank wall using $4\text{M}10$ masonry anchors, which are non-removable.

ELEVATION PLAN SECTION

N.T.S.

A horizontal number line starting at 0 and ending at 1.2 m. The line is divided into six equal segments by tick marks. The labels 0, 0.2, 0.4, 0.6, 0.8, 1.0, and 1.2 m are positioned above the line.

0 , 0.25 , 0.5 , 0.75 , 1.0 , 1.25 , 1.5m



SILVESTER REILLY

Sydney Office –

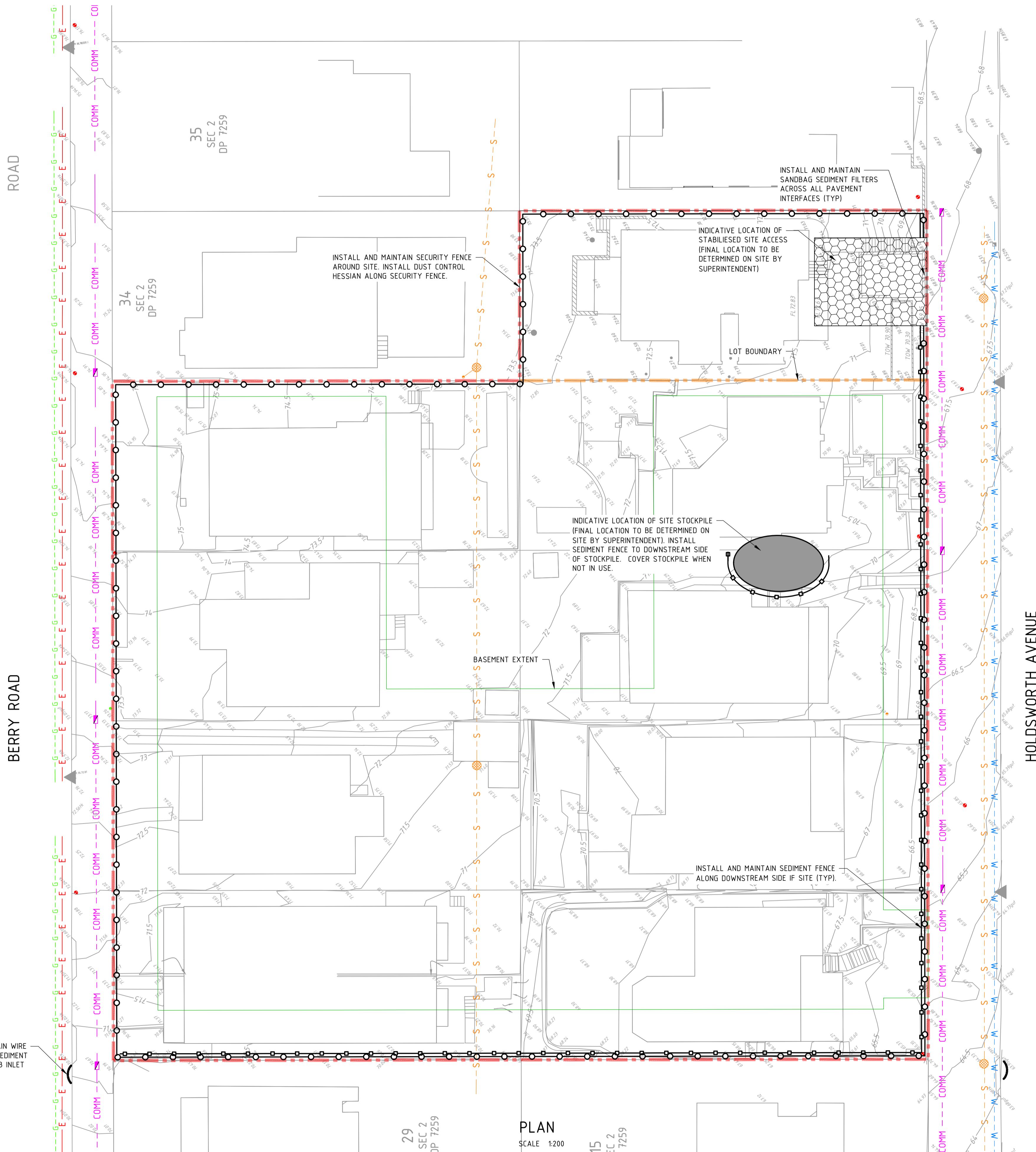
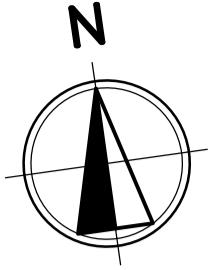
BG
& GE

PROJECT
12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA

STATUS	ISSUED FOR APPROVAL			
	NOT TO BE USED FOR CONSTRUCTION			
DRAWN	DESIGNED	CHECKED	APPROVED	
FM	AM	SH		
DATUM	GRID GDA2020 MGA-56	SCALE	AS SHOWN	
AHD			AT A	

TITLE

OSD SECTIONS AND DETAILS



NOTES

- REFER DRAWING CI-0710 FOR EROSION AND SEDIMENT CONTROL DETAILS.
- CONTRACTOR TO ENSURE SITE DRAINAGE IS NOT ADVERSELY IMPACTED DURING CONSTRUCTION.
- CONTRACTOR TO PROVIDE 'SANDBAG SEDIMENT TRAP' TO ALL PAVED / ROAD AREAS (BOTH PROPOSED AND EXISTING) IN ACCORDANCE WITH THE 'BLUE BOOK'.
- CONTRACTOR TO PROVIDE 'GEOTEXTILE INLET FILTER TRAPS' TO ALL STORMWATER DRAINAGE INLETS (BOTH PROPOSED AND EXISTING) IN ACCORDANCE WITH THE 'BLUE BOOK'.
- INSTALL AND MAINTAIN SANDBAG FILTERS ACROSS ALL PAVEMENT INTERFACES.

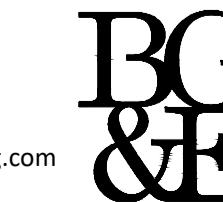
B	14.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH
A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH
REV	DATE	DESCRIPTION	RVD

REVISIONS

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**FULLERSTERRELL**

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L2, 8 Windmill St
Sydney NSW 2000
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PROJECT
12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA

STATUS
ISSUED FOR APPROVAL
NOT TO BE USED FOR CONSTRUCTION

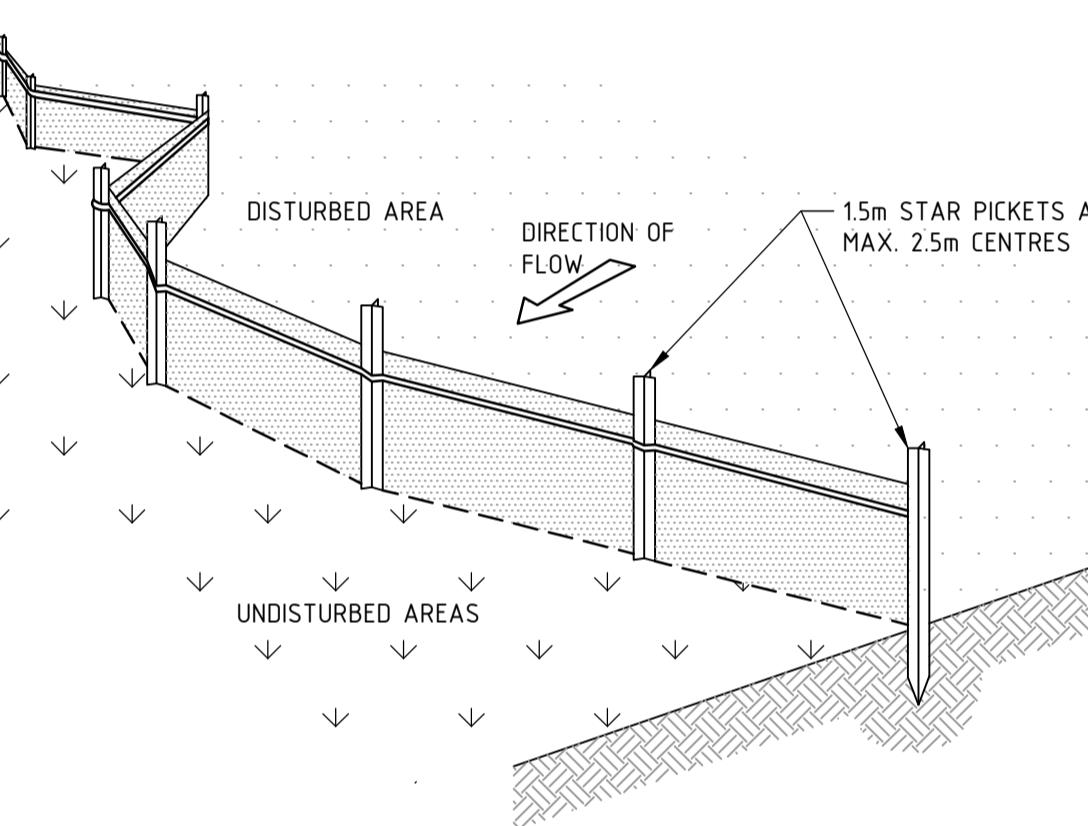
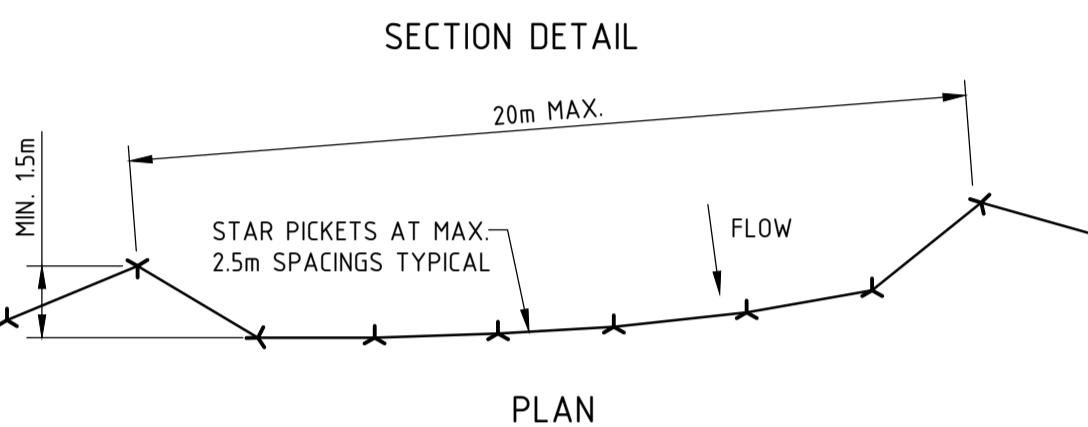
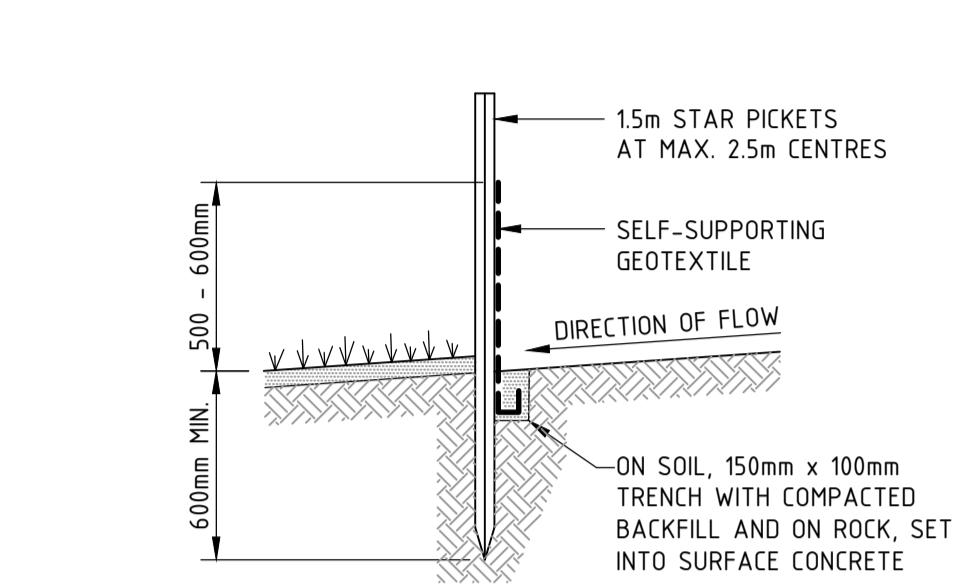
DRAWN BY	DESIGNED BY	CHECKED BY	APPROVED BY
FM	AM	SH	

DATUM: AHD GRID: GDA2020 MGA-56 SCALE: 1:200

TITLE
EROSION AND SEDIMENT CONTROL PLAN

PROJECT No: S21268 DRAWING No: CI-0700 REV: B

SCALE 1:200 AT A1 SIZE

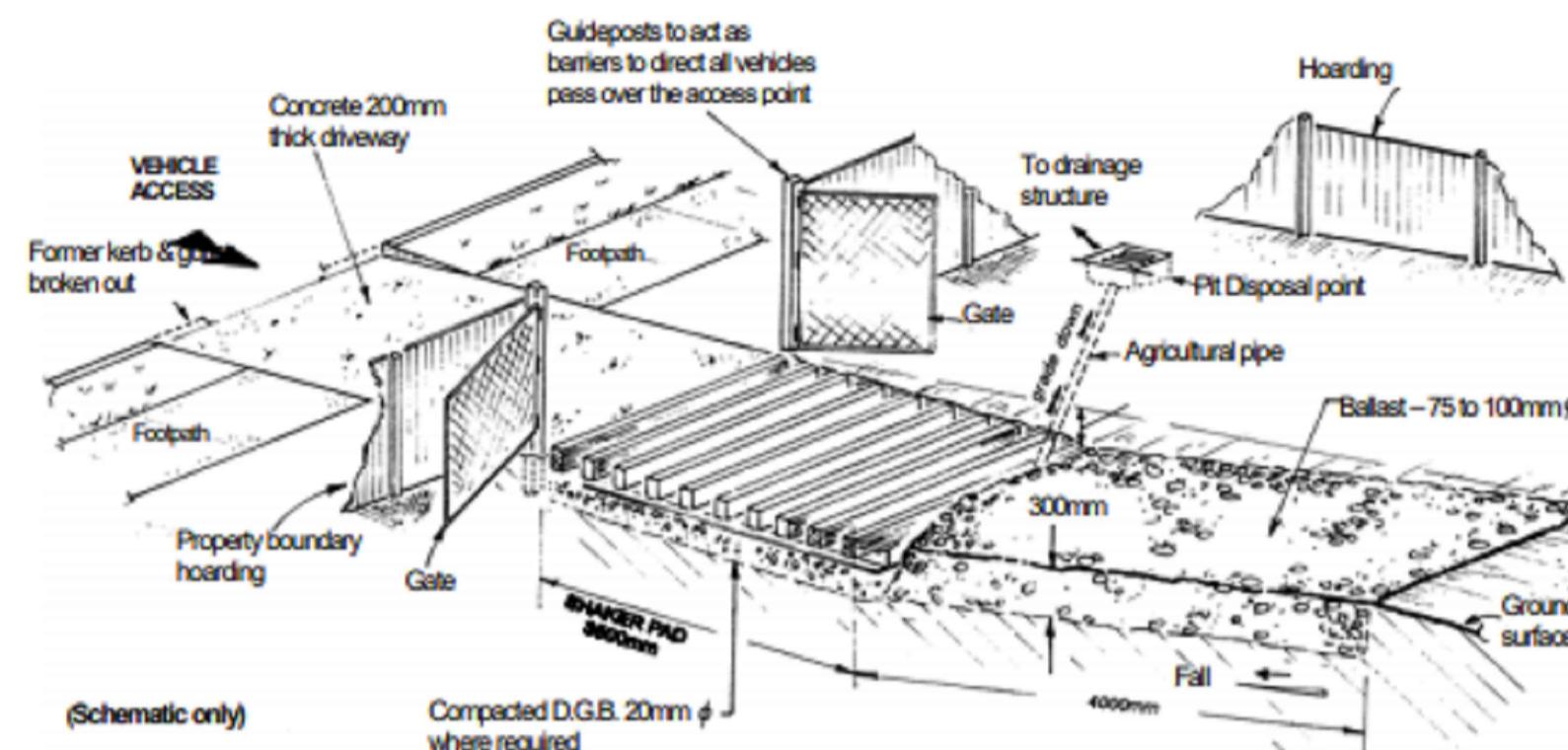


SEDIMENT FENCE CONSTRUCTION NOTES:

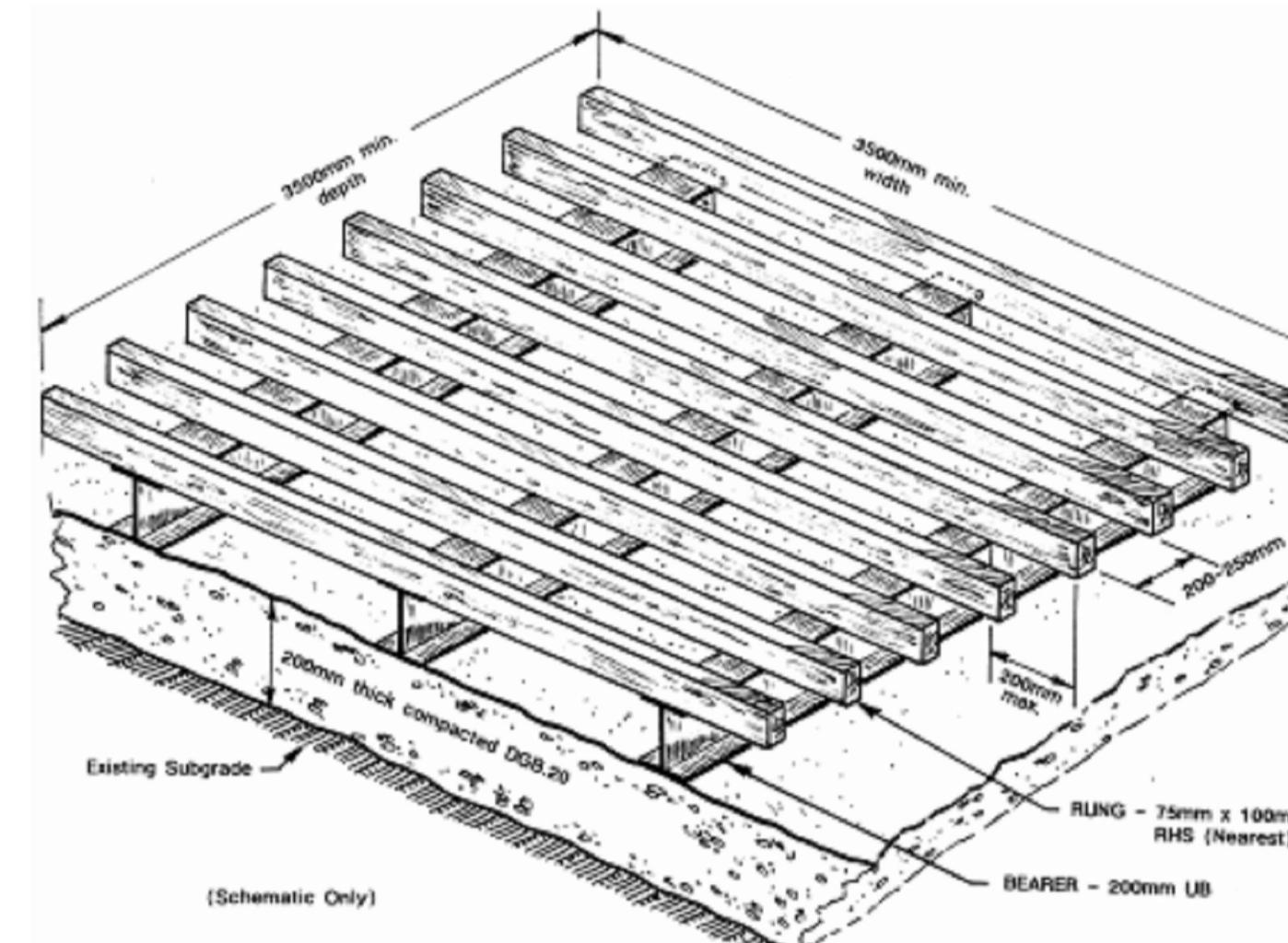
1. CONSTRUCT SEDIMENT FENCES AS CLOSE AS POSSIBLE TO BEING PARALLEL TO THE CONTOURS OF THE SITE, BUT WITH SMALL RETURNS AS SHOWN IN THE DRAWING TO LIMIT THE CATCHMENT AREA OF ANY ONE SECTION. THE CATCHMENT AREA SHOULD BE SMALL ENOUGH TO LIMIT WATER FLOW IF CONCENTRATED AT ONE POINT TO 50 LITRES PER SECOND IN THE DESIGN STORM EVENT, USUALLY THE 10-YEAR EVENT.
2. CUT A 150mm DEEP TRENCH ALONG THE UPSLOPE LINE OF THE FENCE FOR THE BOTTOM OF THE FABRIC TO BE ENTRENCHED.
3. DRIVE 15m LONG STAR PICKETS INTO GROUND AT 2.5m INTERVALS (MAX) AT THE DOWNSLOPE EDGE OF THE TRENCH. ENSURE ANY STAR PICKETS ARE FITTED WITH SAFETY CAPS.
4. FIX SELF-SUPPORTING GEOTEXTILE TO THE UPSLOPE SIDE OF THE POSTS ENSURING IT GOES TO THE BASE OF THE TRENCH. FIX THE GEOTEXTILE WITH WIRE TIES OR AS RECOMMENDED BY THE MANUFACTURER. ONLY USE GEOTEXTILE SPECIFICALLY PRODUCED FOR SEDIMENT FENCING. THE USE OF SHADE CLOTH FOR THIS PURPOSE IS NOT SATISFACTORY.
5. JOIN SECTIONS OF FABRIC AT A SUPPORT POST WITH A 150mm OVERLAP.
6. BACKFILL THE TRENCH OVER THE BASE OF THE FABRIC AND COMPACT IT THOROUGHLY OVER THE GEOTEXTILE.

SEDIMENT FENCE

SCALE N.T.S.



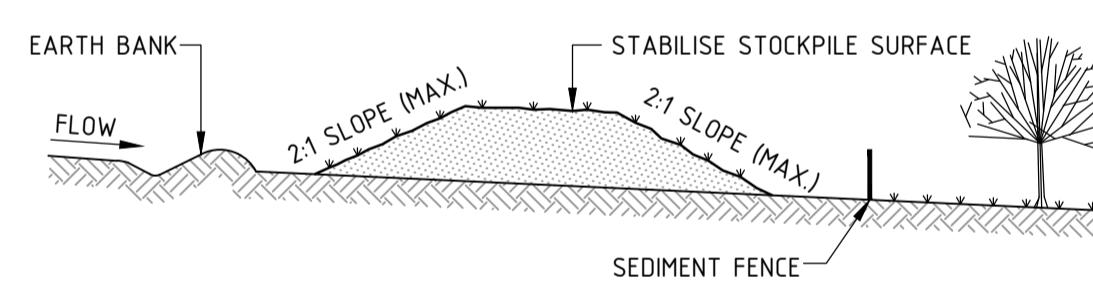
(Schematic only)



(Schematic Only)

STABILISED SITE ACCESS - SHAKER GRID

SCALE N.T.S.

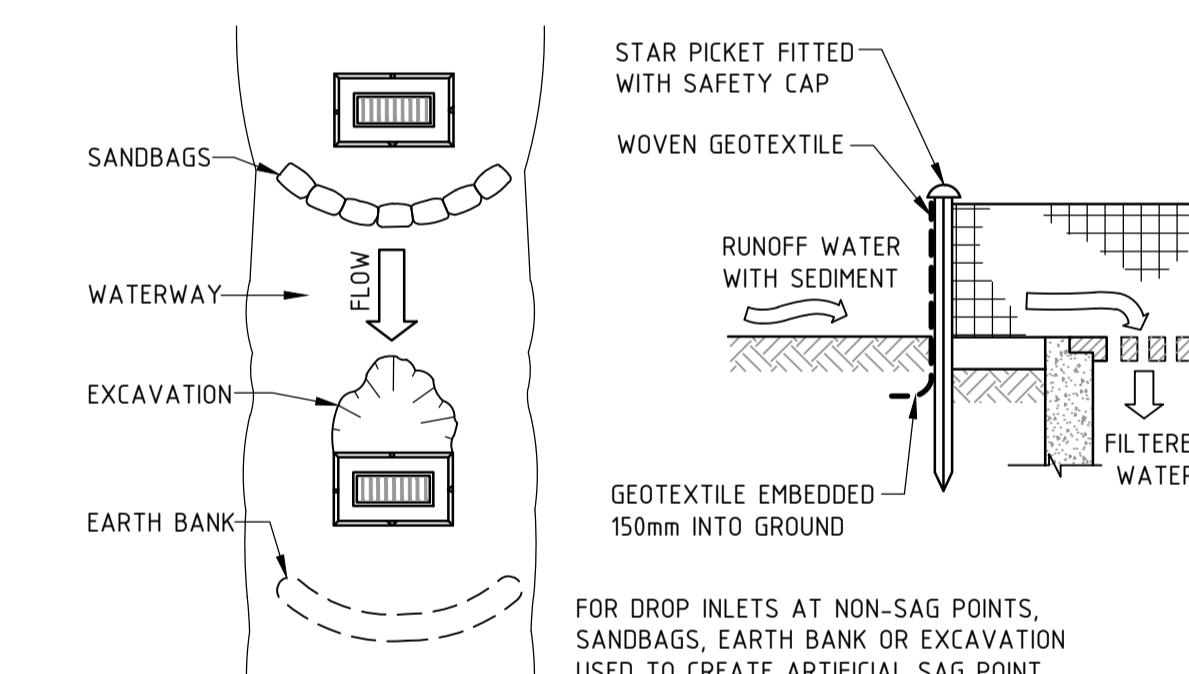
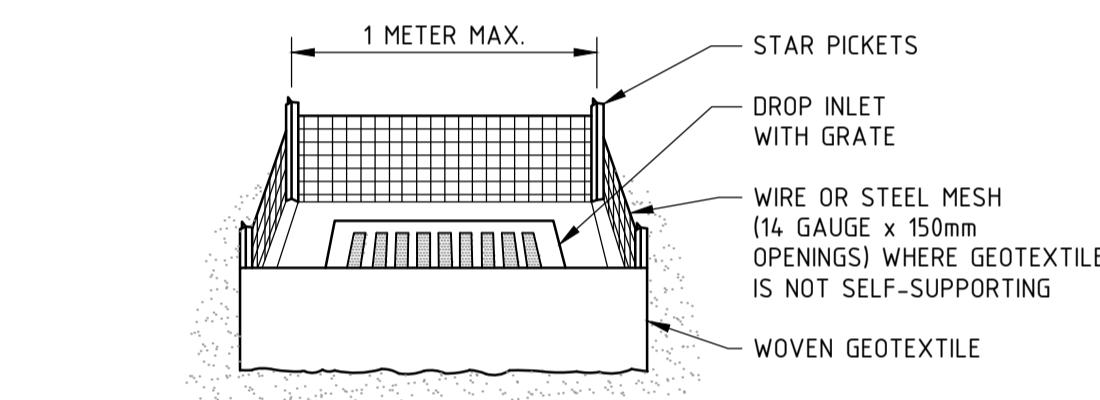


STOCKPILE CONSTRUCTION NOTES:

1. PLACE STOCKPILES MORE THAN 2 (PREFERABLY 5) METRES FROM EXISTING VEGETATION, CONCENTRATED WATER FLOW, ROADS AND HAZARD AREAS.
2. CONSTRUCT ON THE CONTOUR AS LOW, FLAT, ELONGATED MOUNDS.
3. WHERE THERE IS SUFFICIENT AREA, TOPSOIL STOCKPILES SHALL BE LESS THAN 2 METRES IN HEIGHT.
4. WHERE THEY ARE TO BE PLACE FOR MORE THAN 10 DAYS, STABILISE FOLLOWING THE APPROVED ESCP OR SWMP TO REDUCE THE C-FACTOR TO LESS THAN 0.10.
5. CONSTRUCT EARTH BANKS ON THE UPSLOPE SIDE TO DIVERT WATER AROUND STOCKPILES AND SEDIMENT FENCES 1 TO 2 METRES DOWNSLOPE.

STOCKPILES

SCALE N.T.S.

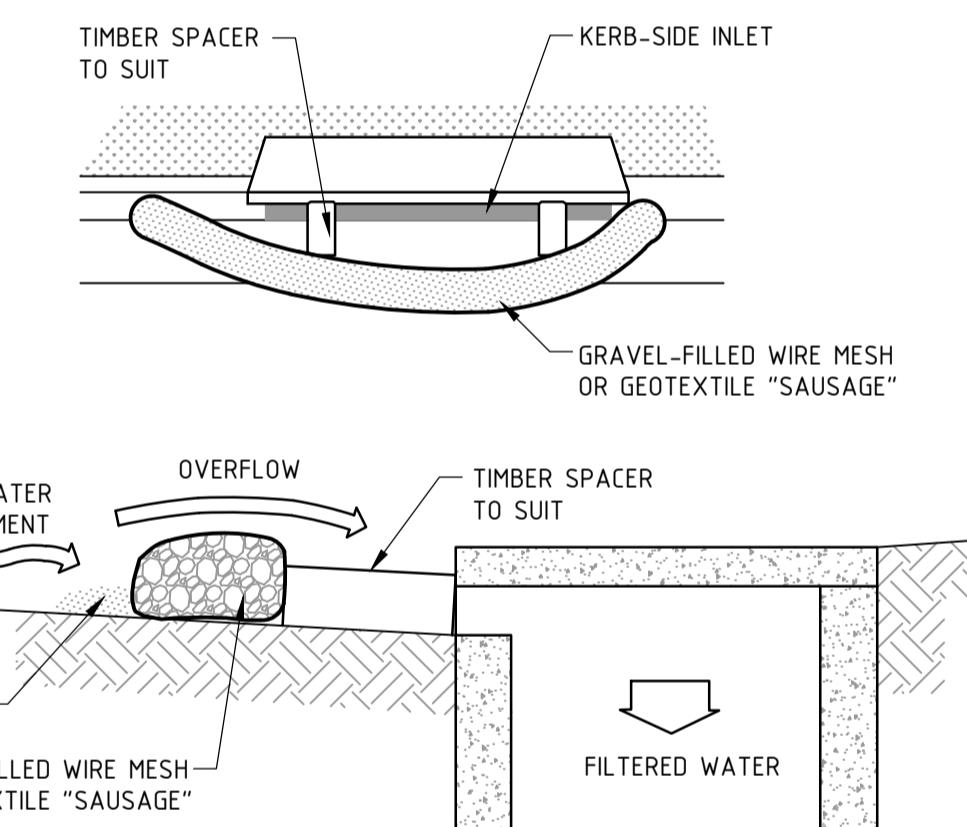


GEOTEXTILE INLET FILTER CONSTRUCTION NOTES:

1. FABRICATE A SEDIMENT BARRIER MADE FROM GEOTEXTILE OR STRAW BALES.
2. PICKET SPACING TO BE A MAXIMUM 1.0m CENTRES.
3. IN WATERWAYS, ARTIFICIAL SAG POINTS CAN BE CREATED WITH SANDBAGS OR EARTH BANKS AS SHOWN IN THE DRAWING.
4. DO NOT COVER THE INLET WITH GEOTEXTILES UNLESS THE DESIGN IS ADEQUATE TO ALLOW FOR ALL WATERS TO BYPASS IT.

GEOTEXTILE INLET FILTER

SCALE N.T.S.



MESH & GRAVEL INLET FILTER CONSTRUCTION NOTES:

1. INSTALL FILTERS TO KERB INLETS ONLY AT SAG POINTS.
2. FABRICATE A SLEEVE MADE FROM GEOTEXTILE OR WIRE MESH LONGER THAN THE LENGTH OF THE INLET PIT AND FILL IT WITH 25MM TO 50MM GRAVEL.
3. FORM AN ELLIPTICAL CROSS-SECTION ABOUT 150mm HIGH x 400mm WIDE.
4. PLACE THE FILTER AT THE OPENING LEAVING AT LEAST A 100mm SPACE BETWEEN IT AND THE KERB INLET. MAINTAIN THE OPENING WITH SPACER BLOCKS.
5. FORM A SEAL WITH THE KERB TO PREVENT SEDIMENT BYPASSING THE FILTER.
6. SANDBAGS FILLED WITH GRAVEL CAN SUBSTITUTE FOR THE MESH OR GEOTEXTILE PROVIDED THEY ARE PLACED SO THAT THEY CAN FIRMLY ABUT EACH OTHER AND SEDIMENT-LADEN WATERS CANNOT PASS BETWEEN.

MESH & GRAVEL INLET FILTER

SCALE N.T.S.

A	09.06.22	ISSUED FOR DEVELOPMENT APPROVAL	SH	RVD	REV	DATE	DESCRIPTION	RVD
REVISIONS							REVISIONS	



**FULLER
SILVESTER**

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BG
&
E
PROJECT
12-20 BERRY ROAD &
11-19 HOLDWORTH AVENUE
ST LEONARDS SOUTH NEW 2065
AUSTRALIA

STATUS
ISSUED FOR APPROVAL
NOT TO BE USED FOR CONSTRUCTION
DRAWN
DESIGNED
CHECKED
APPROVED
FM
AM
SH

DATUM
AHD
GRID
GDA2020
MGA-56
SCALE
NOT TO SCALE
AT A1 SIZE

TITLE
**EROSION AND SEDIMENT
CONTROL DETAILS**

PROJECT No.
S21268
DRAWING No.
CI-0710
REV
A

APPENDIX C

Council OSD Tank Detention Calculation Sheet

Appendix 13 – OSD Checklist for DA Submission

ON-SITE STORMWATER DETENTION CHECKLIST

This form is to be used to determine if OSD will be required for residential developments and must be completed before the submission of any application.



PART A. Address and type of proposed development

Street No..... Street Name. 12-20 Berry Road & 11-19 Holdsworth Ave
Lot No..... DP No..... Suburb. St Leonards South

Type of development (tick relevant box).

- | | | | |
|--------------------------|-----------------------------|-------------------------------------|---|
| <input type="checkbox"/> | Duplex Residential Building | <input checked="" type="checkbox"/> | Multiple Occupancy Residential (villa, flats etc) |
| <input type="checkbox"/> | Extensions | <input type="checkbox"/> | Single Residential |
| <input type="checkbox"/> | Commercial | <input type="checkbox"/> | Garages |
| <input type="checkbox"/> | Other..... | | |

PART B. Exemption for discharge directly to Lane Cove River

Is the site within the designated exclusion zone along the foreshore of the Lane Cove River. (tick one only).
(Confirm with Council's Urban Services Division).

- No Yes

If yes, OSD is not required, If no go to part C

PART C. Exemption for minimum allowable size of site impervious area

- | | |
|--|-------|
| (a) Site Area | m^2 |
| (b) Existing impervious area to be removed | m^2 |
| (c) Existing impervious to be retained | m^2 |
| (d) Proposed new impervious area: | |
| (d1) Roof area | m^2 |
| (d2) Driveways | m^2 |
| (d3) Other paved area | m^2 |
| (d4) Supplementary areas (i.e Pervious paving area x 25%) | m^2 |
| (e) Total proposed NEW impervious area (d1) + (d2) + (d3) + (d4) - (b) = | m^2 |
| (f) Total post development impervious area (c) + (d1) + (d2) + (d3) + (d4) = | m^2 |
| (g) Post development impervious area (f) x 100 / (a) = | % |

OSD will not be required if one or more of the following are satisfied

- (e) is less than 50m² increase in site cover and (f) is less than 65% of the total site area.
(only applicable for alterations and additions)
- (g) is less than 35% of site area

Note: If OSD is not required, then the collected stormwater runoff is to be directed to a 600x600mm environmental pollution control pit with sediment collection sump and drainage filter, prior to discharging to an approved outlet. The control pit is to be designed as a gross pollutant trap to remove pollutants from the stormwater flow.

PART D. Special Consideration

Where the applicant believes that special consideration should be given for exemption from OSD, even though Parts A, B, C, or D are not satisfied, they may request exemption from OSD. Consideration may only be given on reasonable grounds and should be discussed with Council's Development Engineer.

Appendix 14 – OSD Calculation Sheet

**ON-SITE DETENTION CALCULATION SHEET****DEVELOPMENT TYPE:** Residential Flat Building**ADDRESS:** 12-20 Berry Road & 11-19 Holdsworth Ave,
St Leonards South

Site Area (m ²)	<u>4,457.6</u>	(A)
Total Impervious Area (roofs, driveways, hardstand etc) (m ²)	<u>3020</u>	(B)
Total Area draining to the Storage Facility (m ²) (impervious and pervious areas)	<u>4,218.6</u>	(C)
New Impervious Area bypassing the Storage Facility	<u>176</u>	(D)
$\frac{(B)+(D)}{(B)} =$	<u>1.06</u>	(E)

cannot be greater than 1.25.

Permitted Site Discharge (PSD) rate per m²If (D) = 0 then PSD = 0.014 l/sec/m²If (D) ≠ 0 then PSD = $0.014 \times (E)^{-1.37}$ l/sec/m² 0.01293 (F)**PERMITTED SITE DISCHARGE (l/s)** (C) x (F)54.54 l/s**Storage Volume per m²**(G) = 0.0255 m³/m² for all Catchments0.0255 (G)**SITE STORAGE REQUIREMENT (m³)** ((C) + (D)) x (G)112.14 m³**OUTLET CONTROL - using a Sharp Edged Orifice Plate**Height Difference between top water level and Centre of Orifice (m) 3 (H)**ORIFICE DIAMETER (mm)**

$$123 \text{ mm} = 21.9 \sqrt{\frac{PSD}{\sqrt{H}}}$$

Should pipe and pit losses be used to control outflow, the calculations are to be attached.